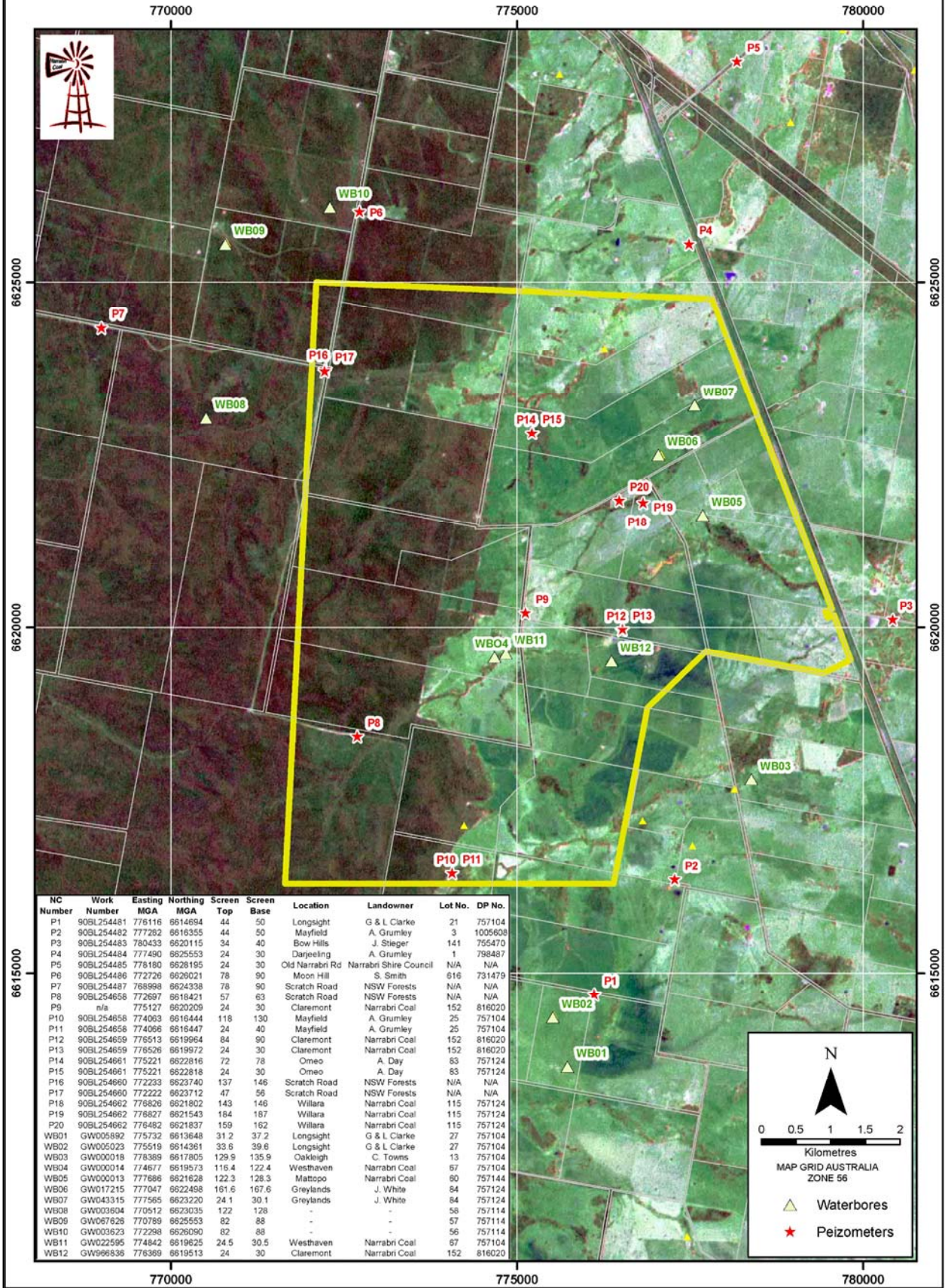


APPENDIX A

NARRABRI WATER LEVEL HYDROGRAPHS

NARRABRI PROJECT - GROUNDWATER MONITORING NETWORK

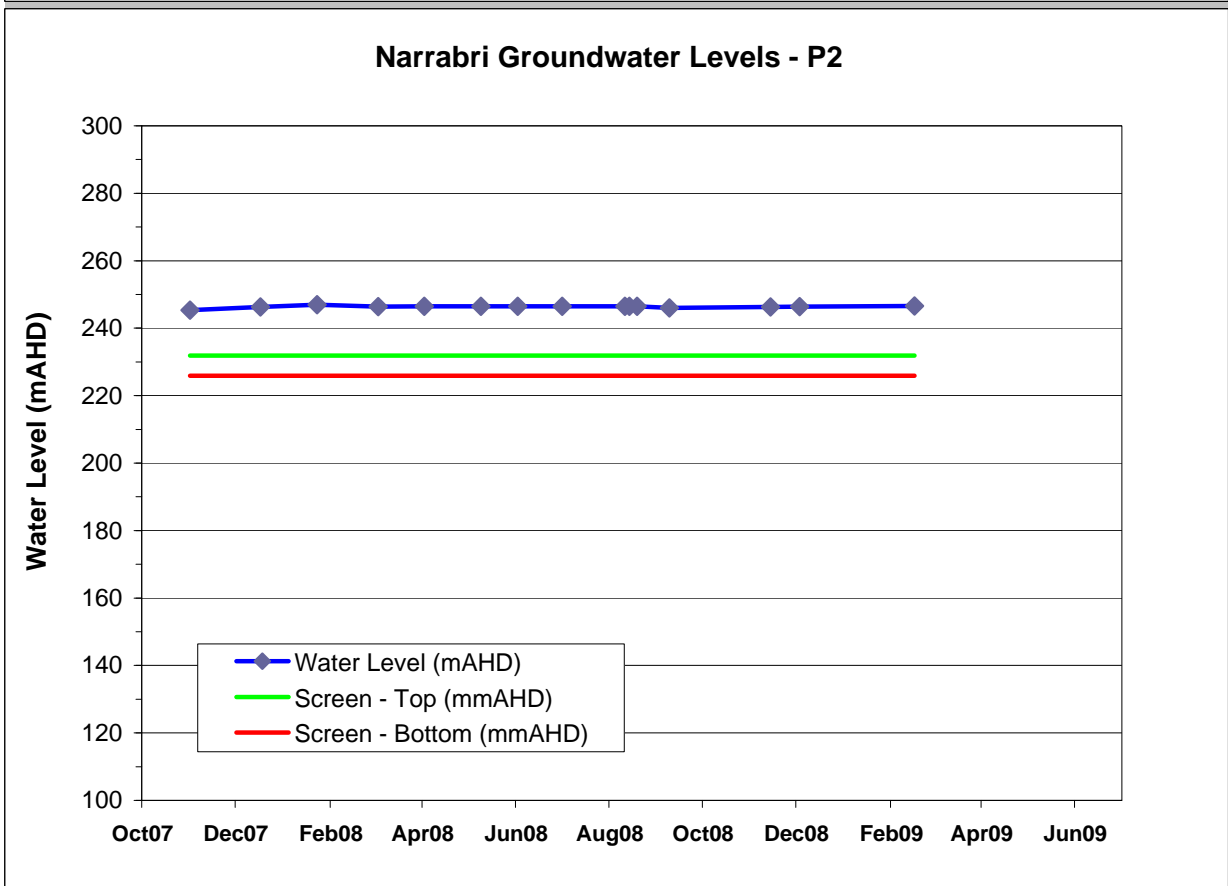
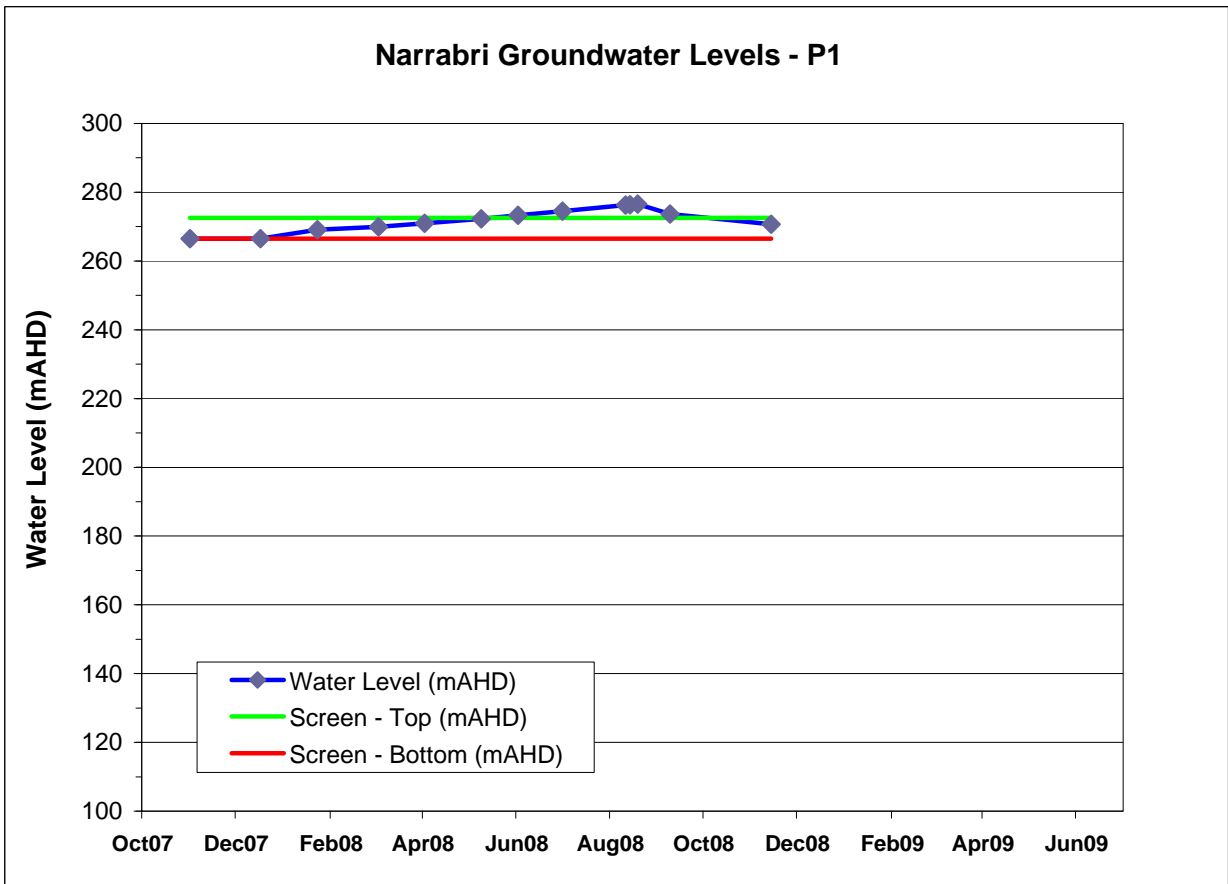


Date: 24 April 2009
 Initials: TL
 Drawing No: S28-051a

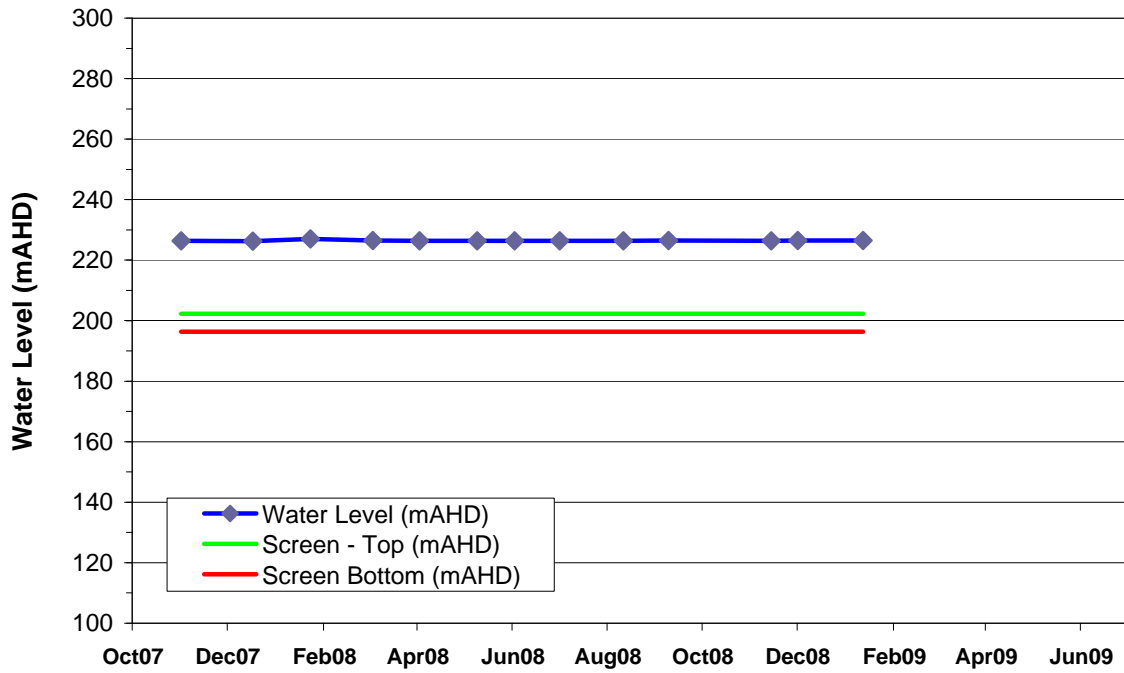
Scale: As Shown
 Job No: S28
 Rev: A

Narrabri Coal Pty Ltd

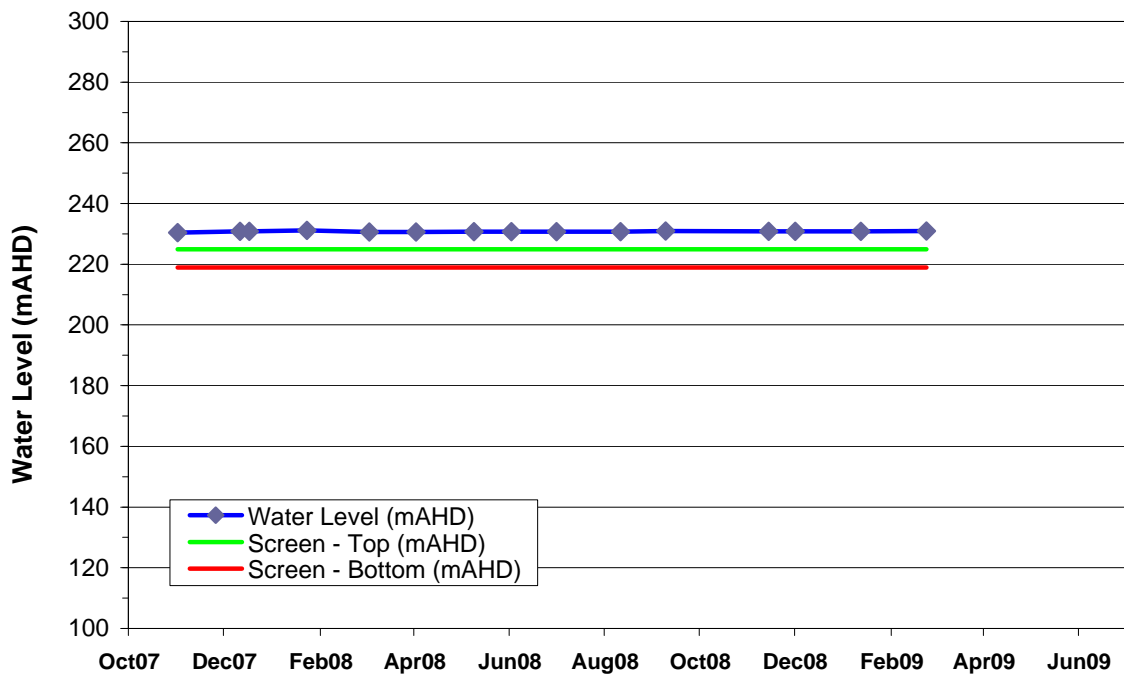
Groundwater Monitoring Location



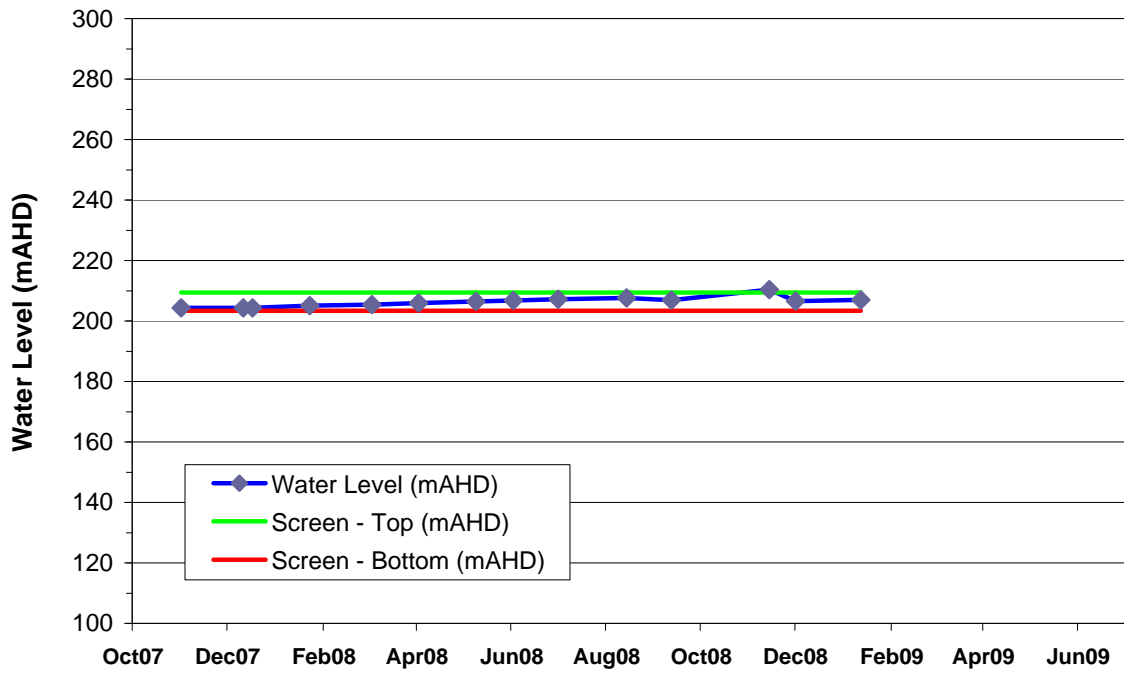
Narrabri Groundwater Levels - P3



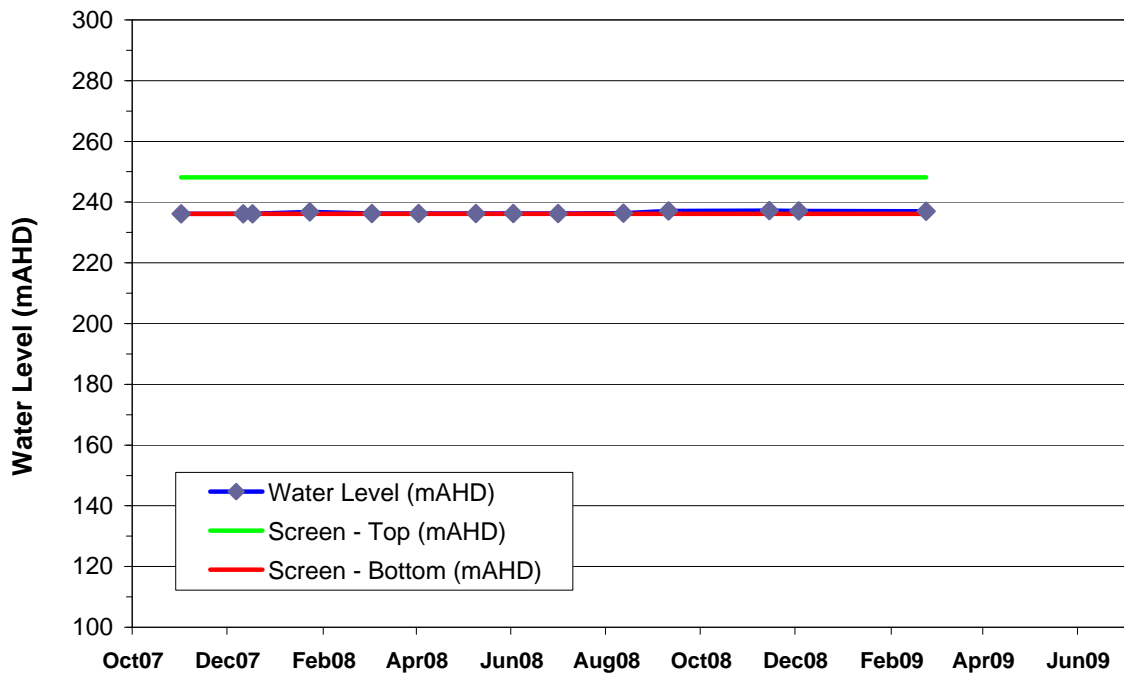
Narrabri Groundwater Levels - P4



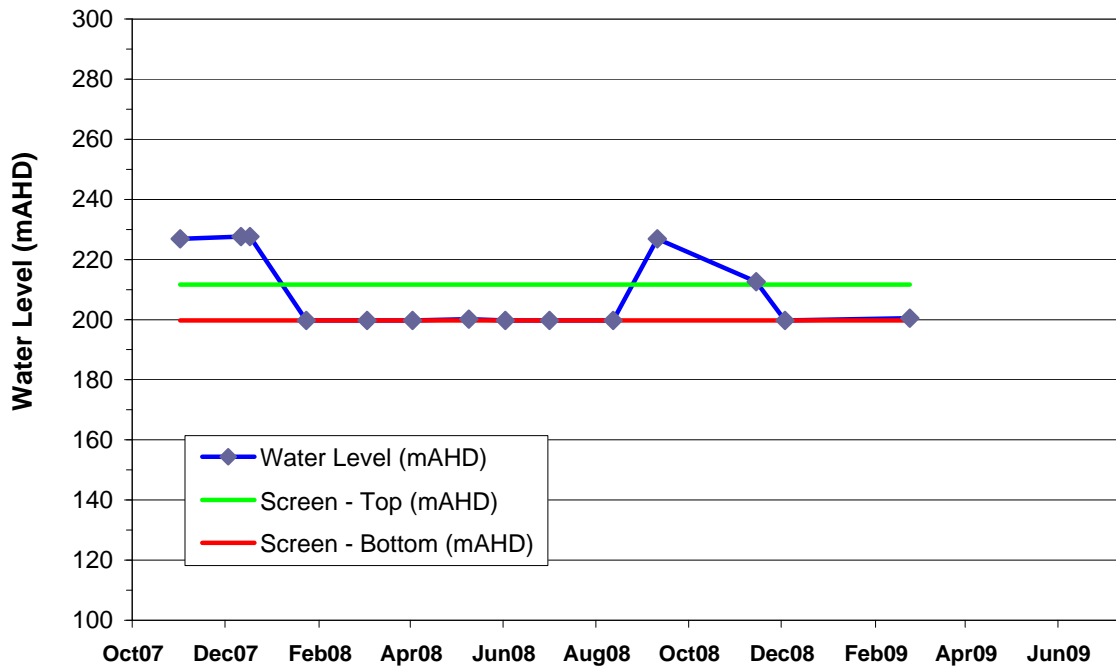
Narrabri Groundwater Levels - P5



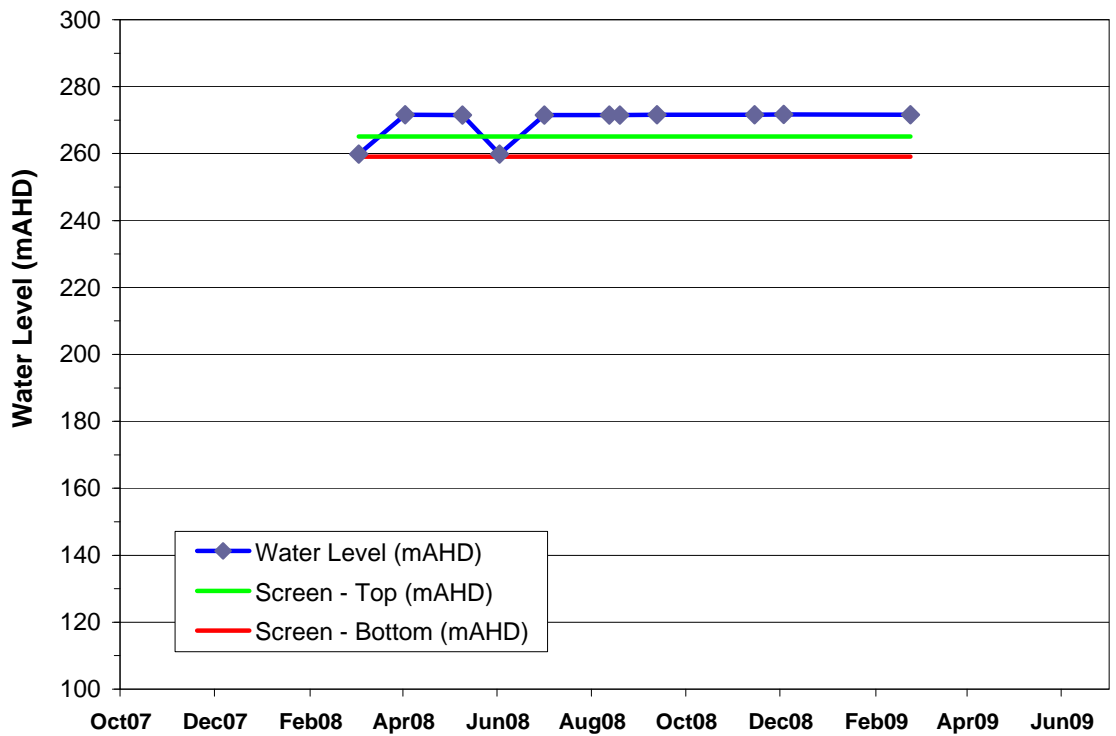
Narrabri Groundwater Levels - P6



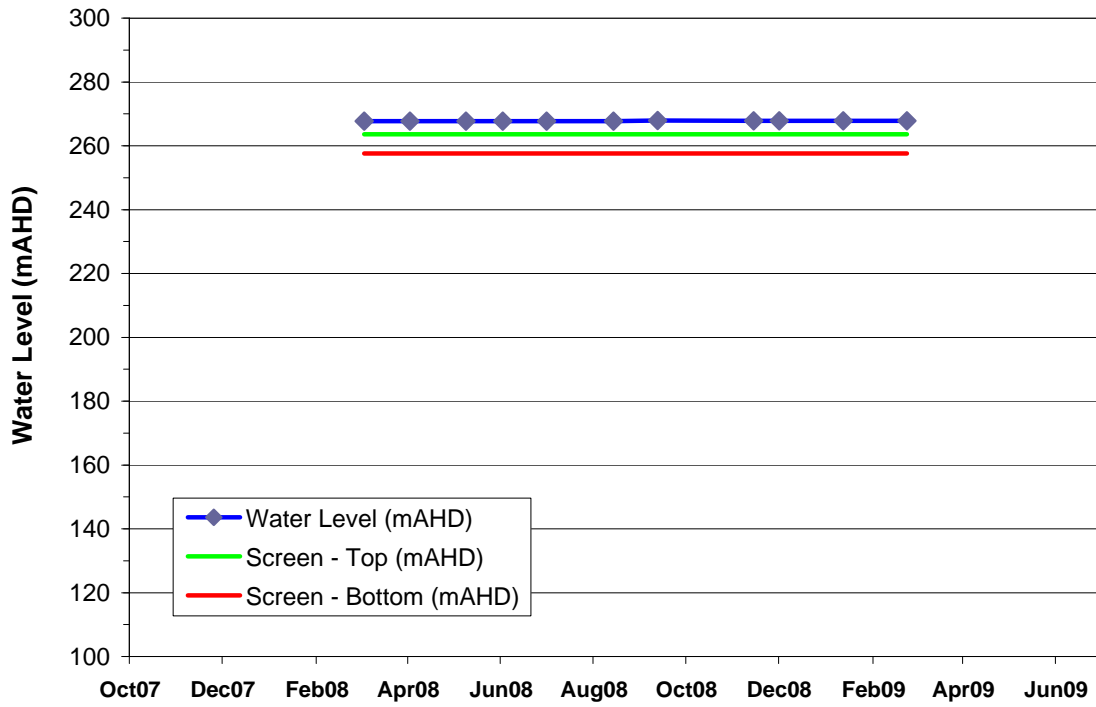
Narrabri Groundwater Levels - P7



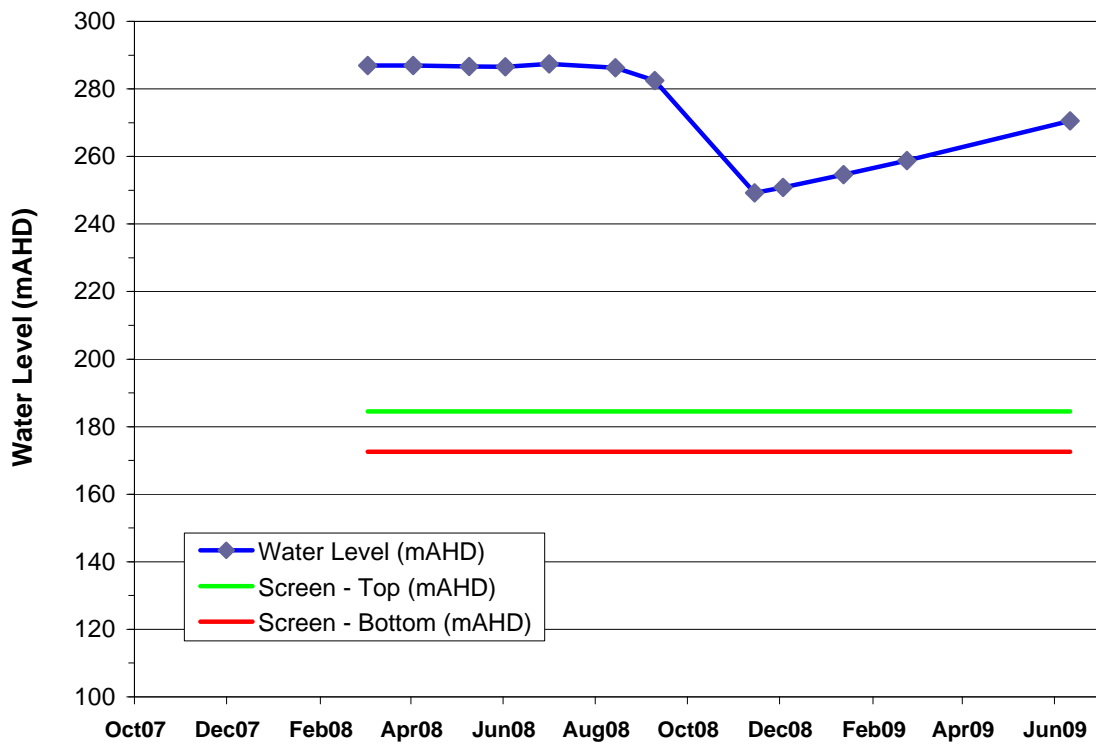
Narrabri Groundwater Levels - P8



Narrabri Groundwater Levels - P9



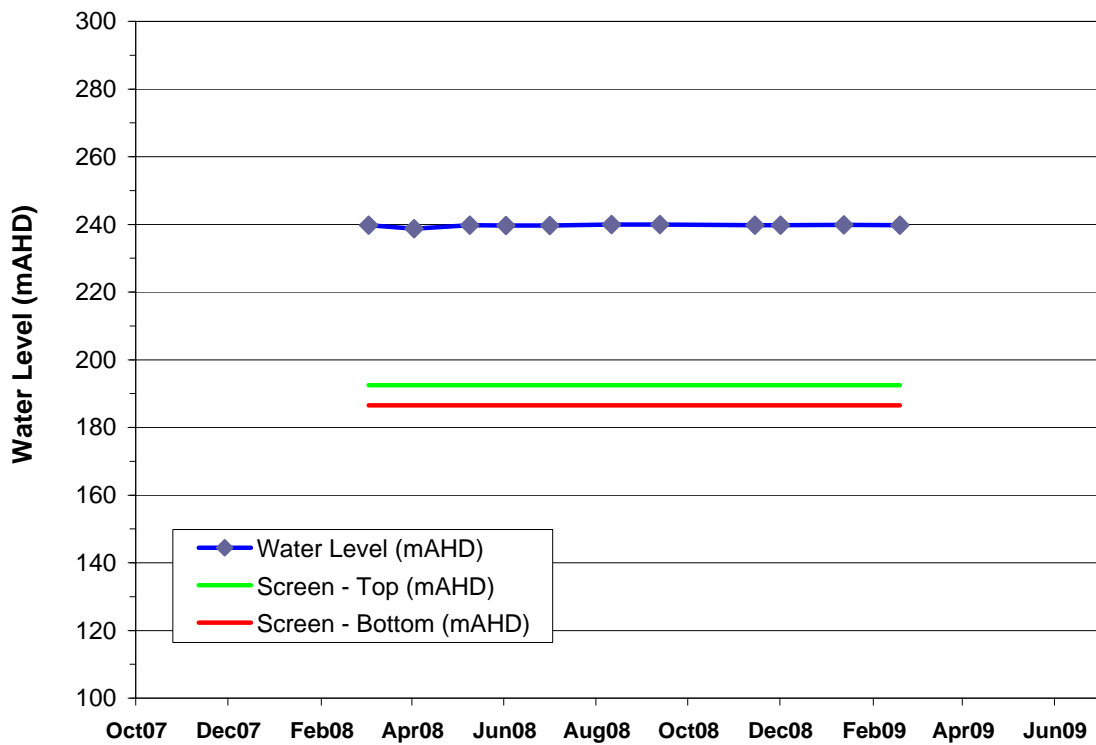
Narrabri Groundwater Levels - P10



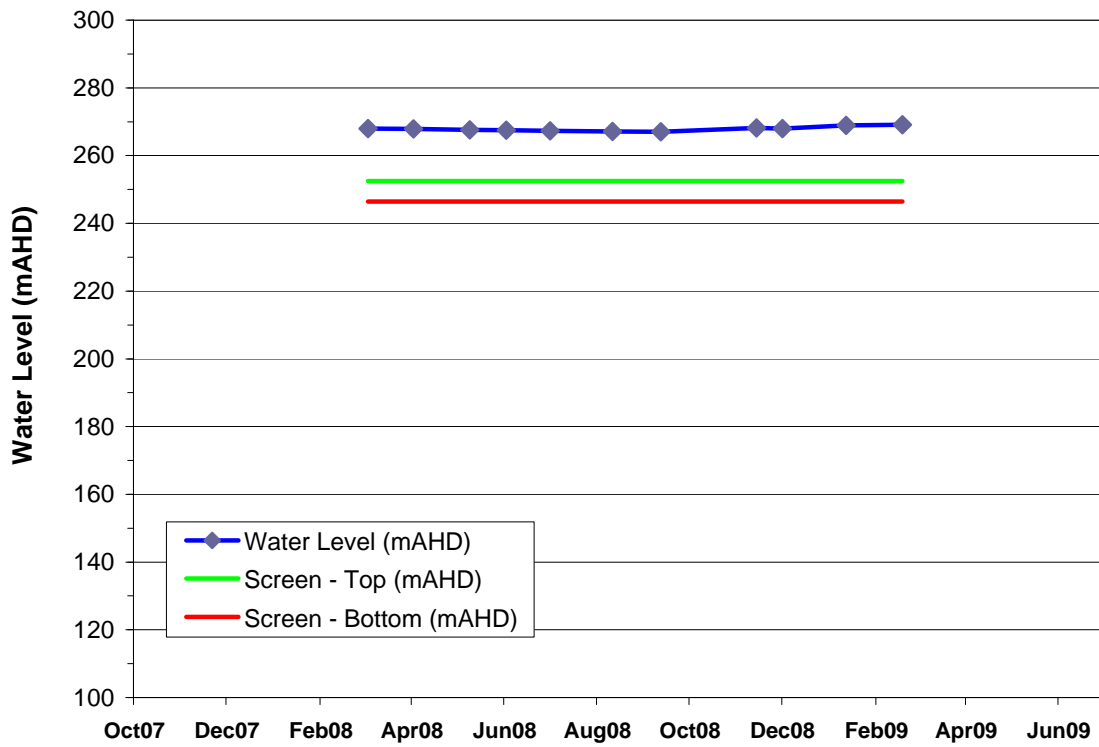
Narrabri Groundwater Levels - P11



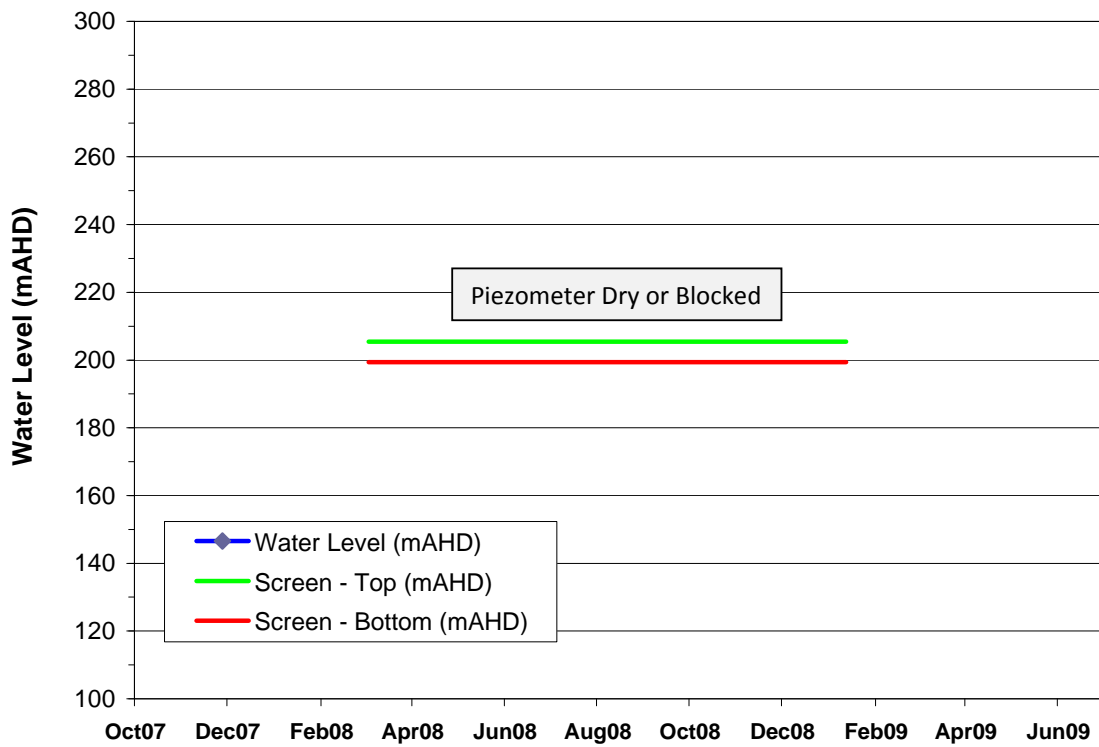
Narrabri Groundwater Levels - P12



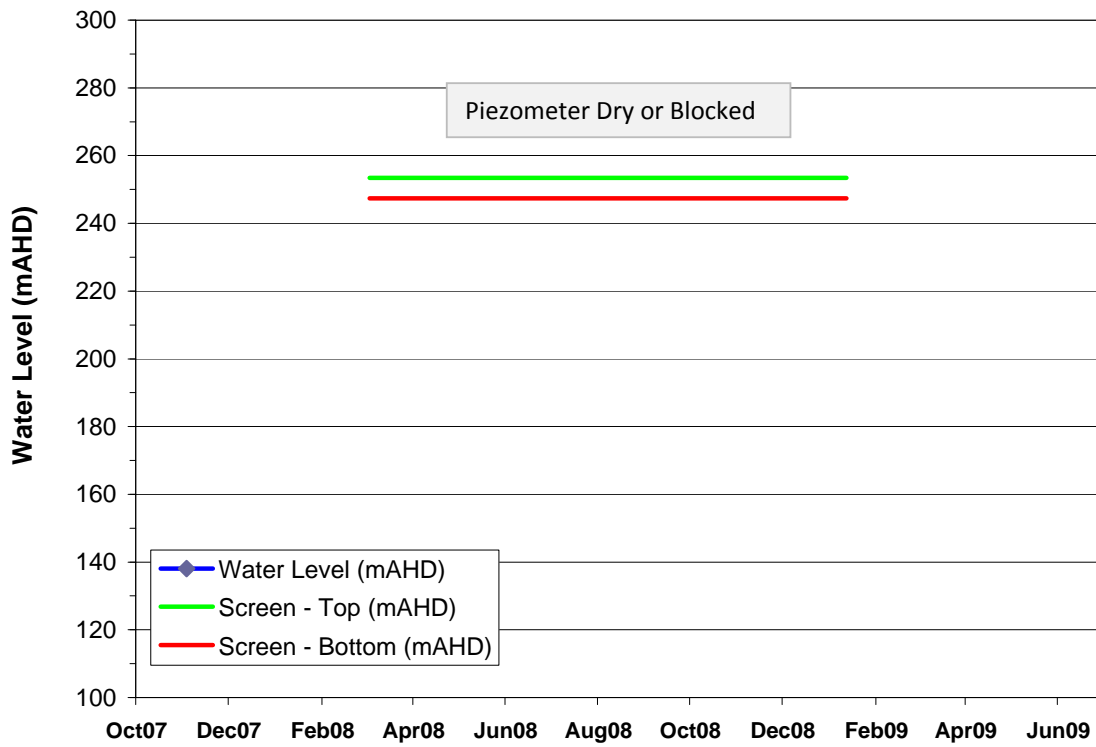
Narrabri Groundwater Levels - P13



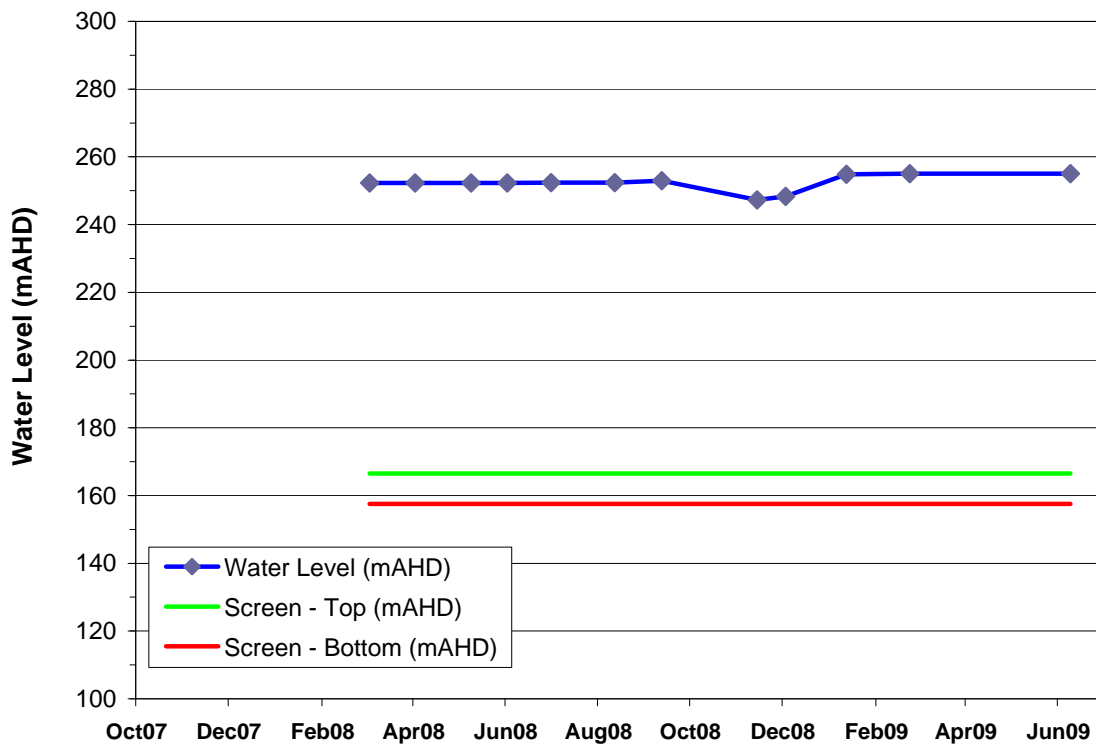
Narrabri Groundwater Levels - P14



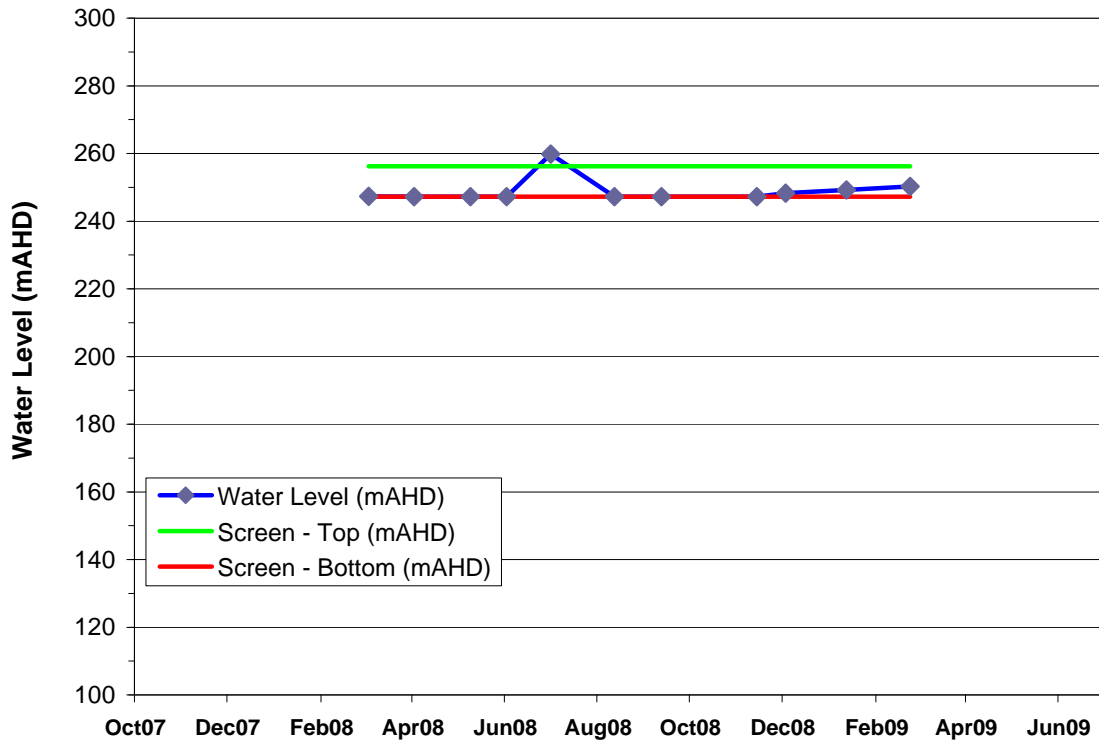
Narrabri Groundwater Levels - P15



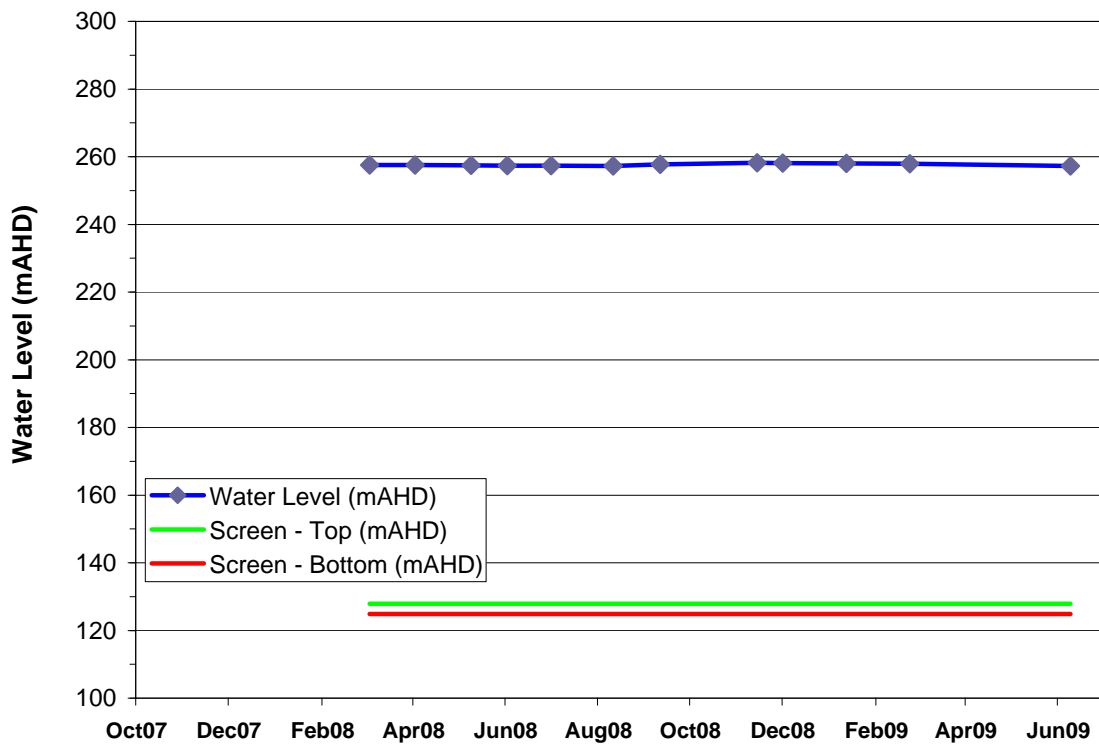
Narrabri Groundwater Levels - P16



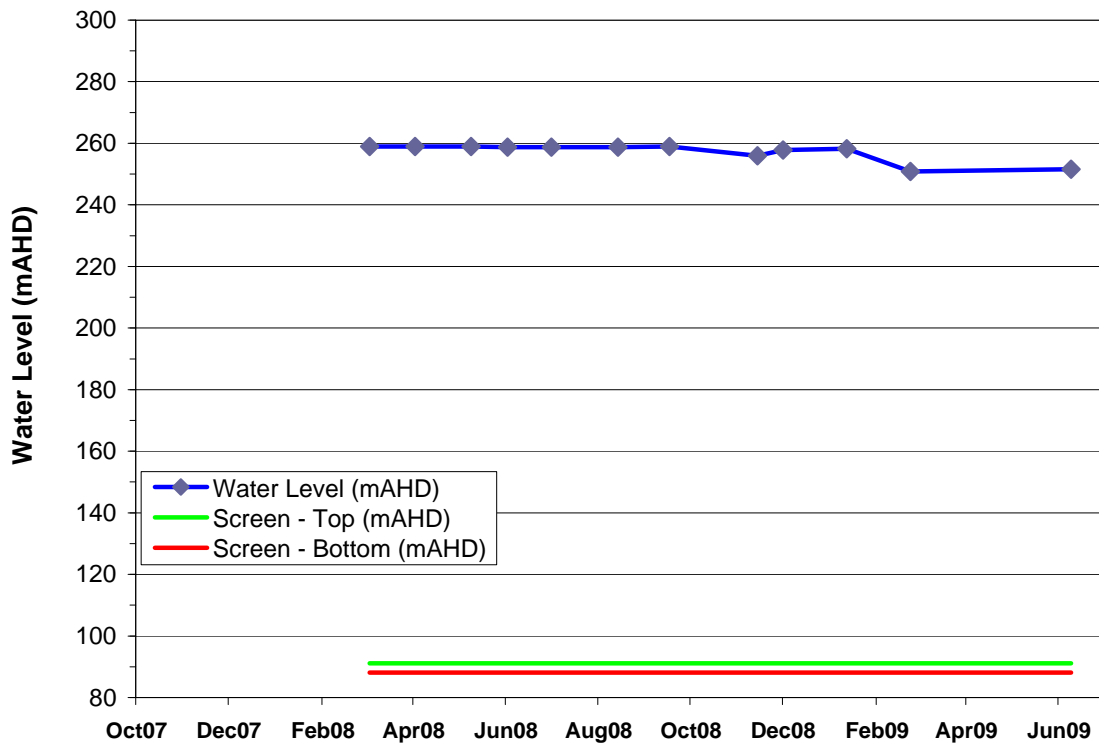
Narrabri Groundwater Levels - P17



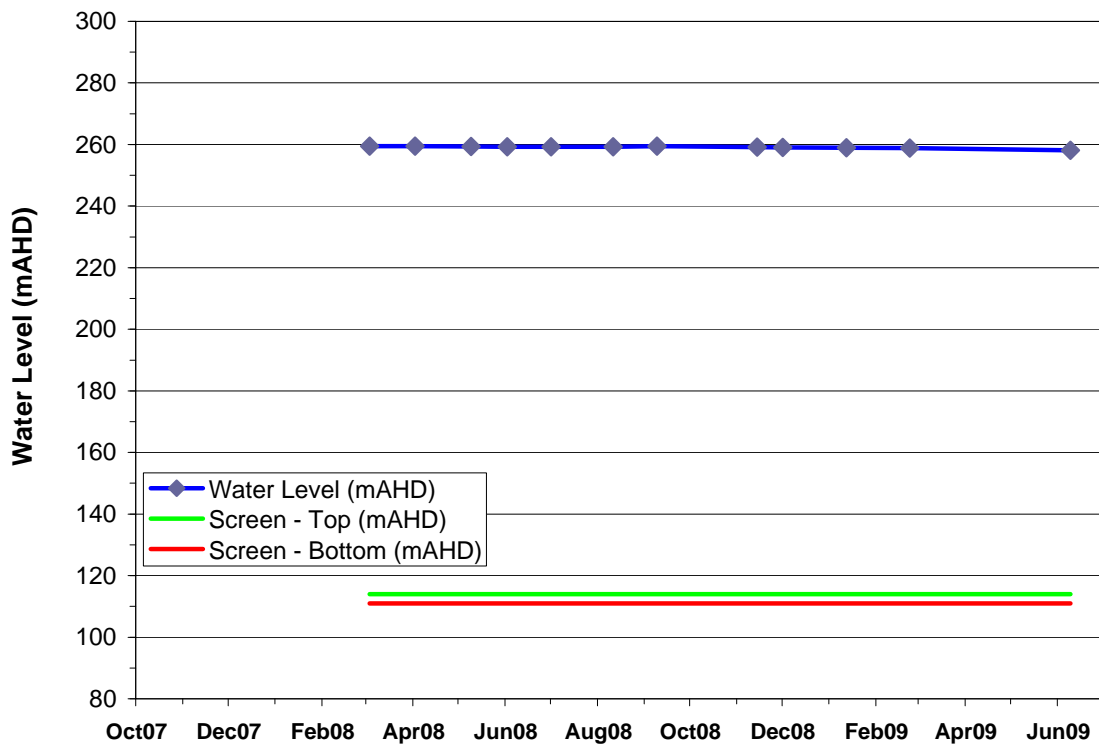
Narrabri Groundwater Levels - P18



Narrabri Groundwater Levels - P19



Narrabri Groundwater Levels - P20

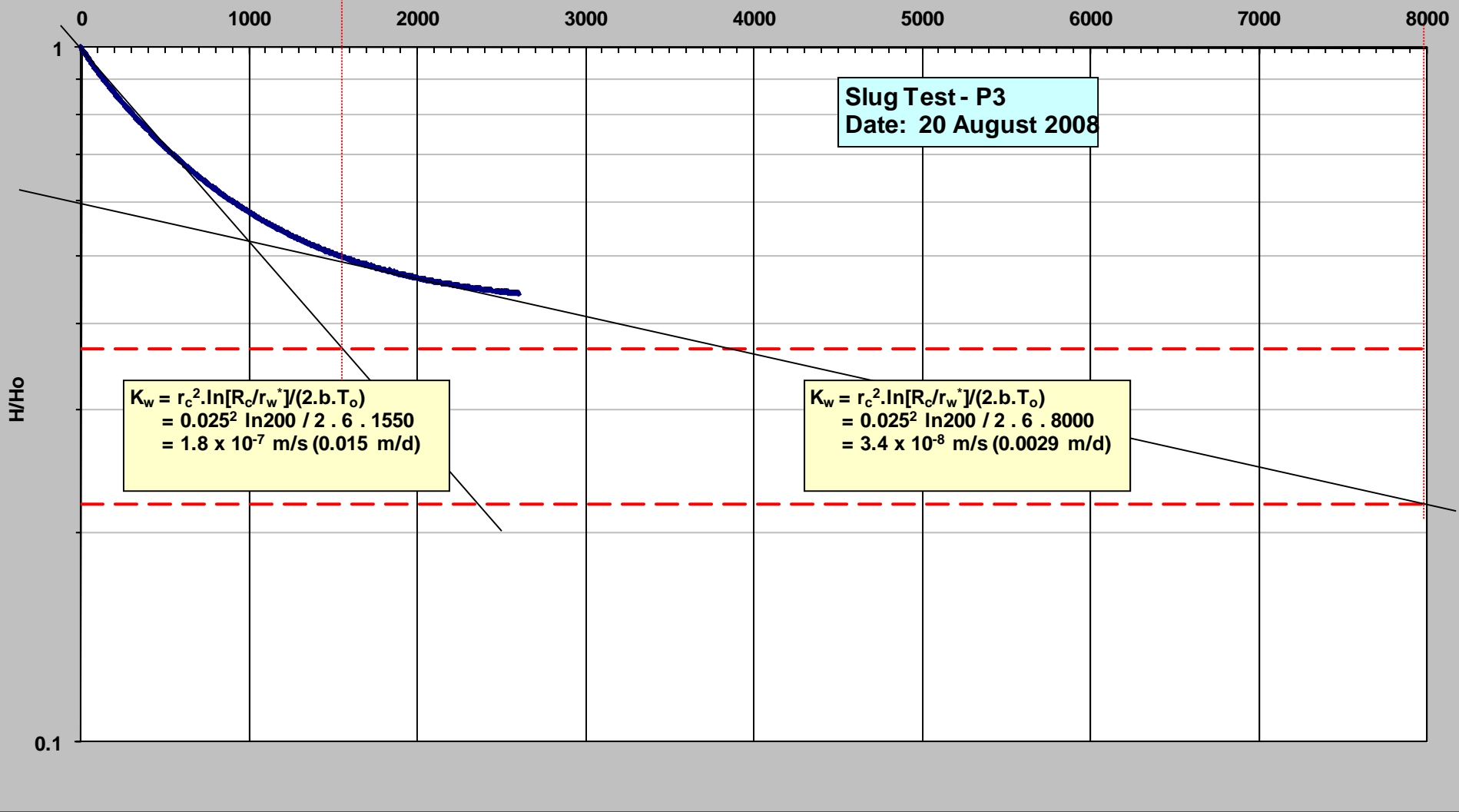


APPENDIX B

NARRABRI HYDRAULIC TEST DATA

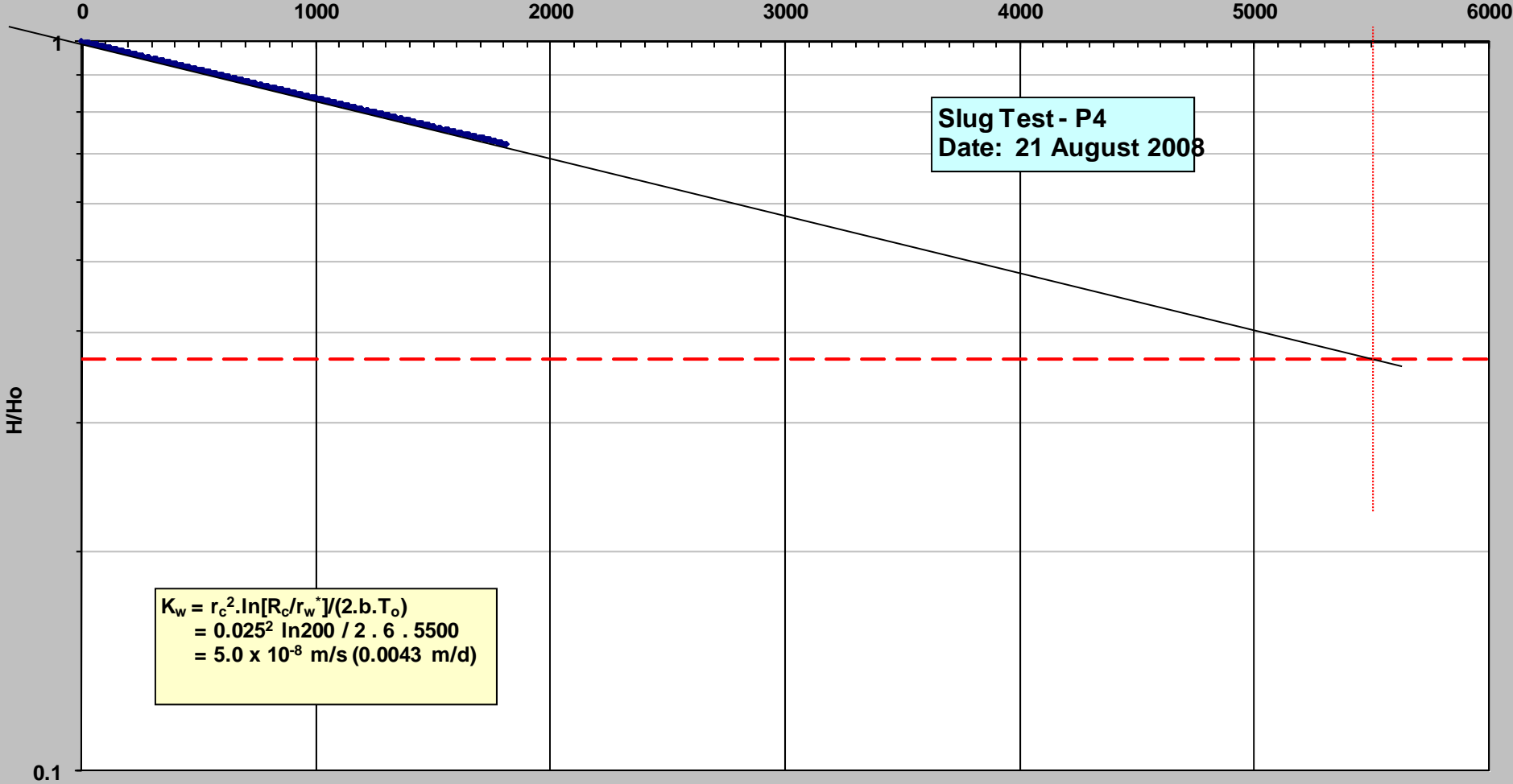
SLUG TEST - P3

Time (sec)



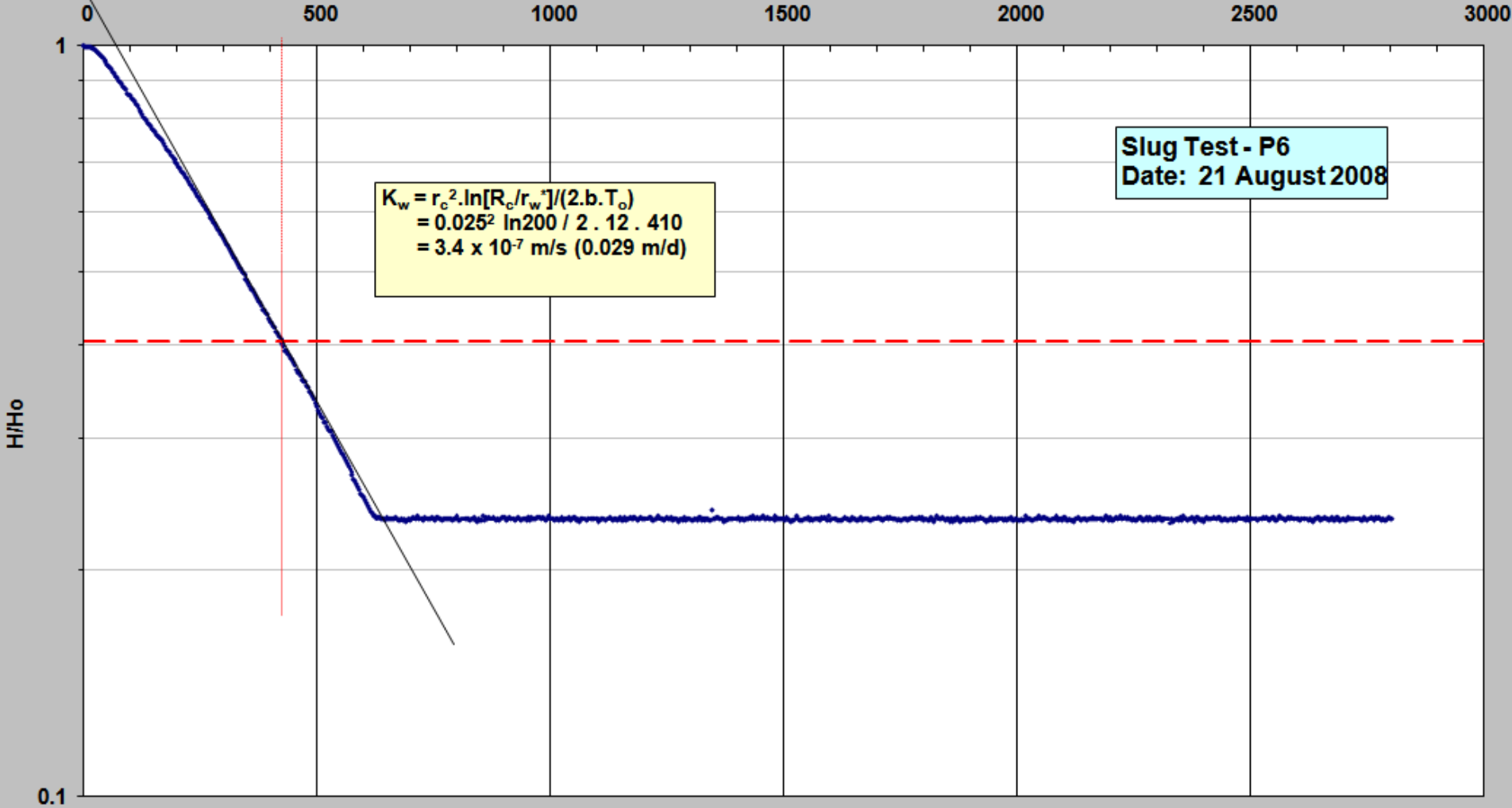
SLUG TEST - P4

Time (sec)



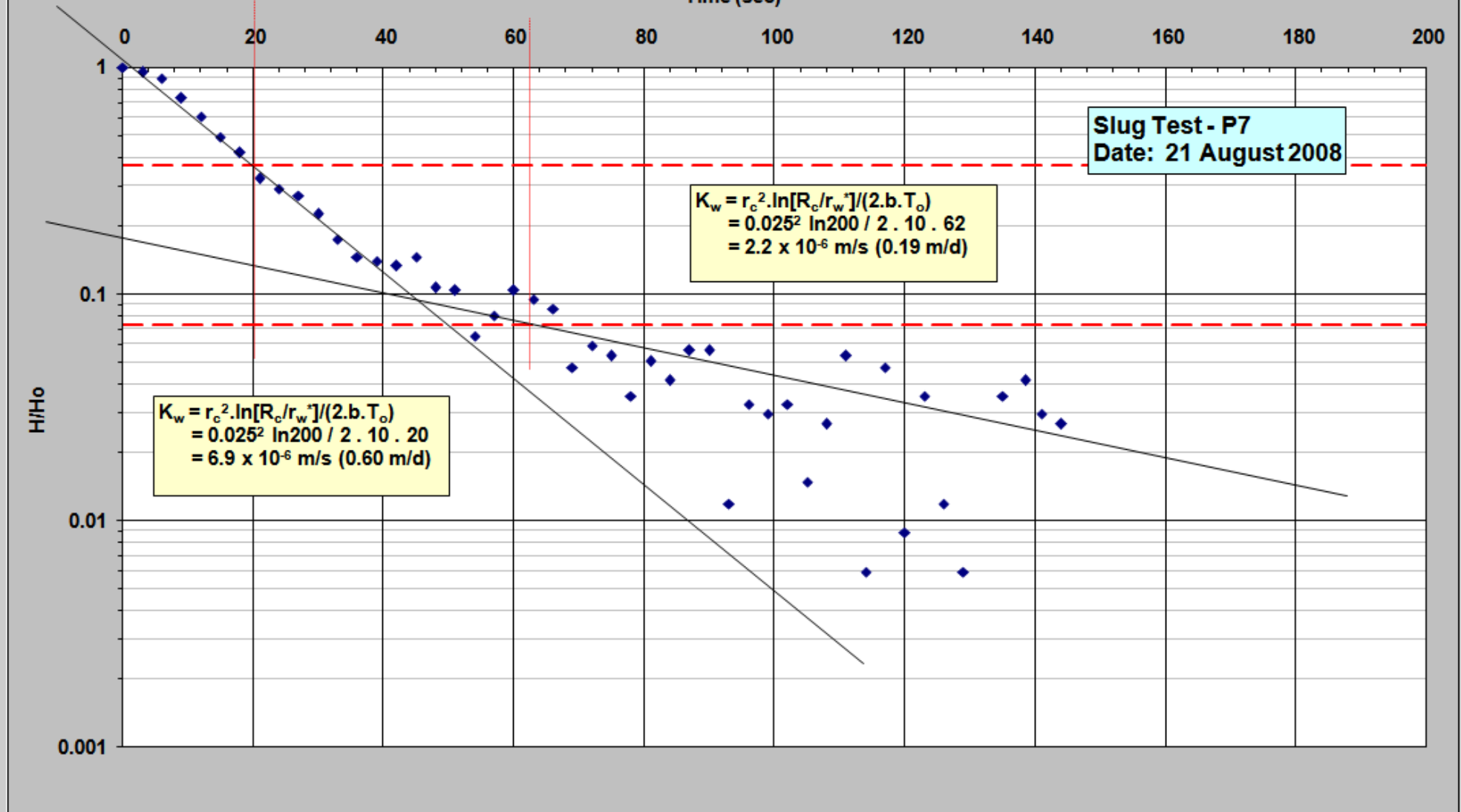
SLUG TEST - P6

Time (sec)

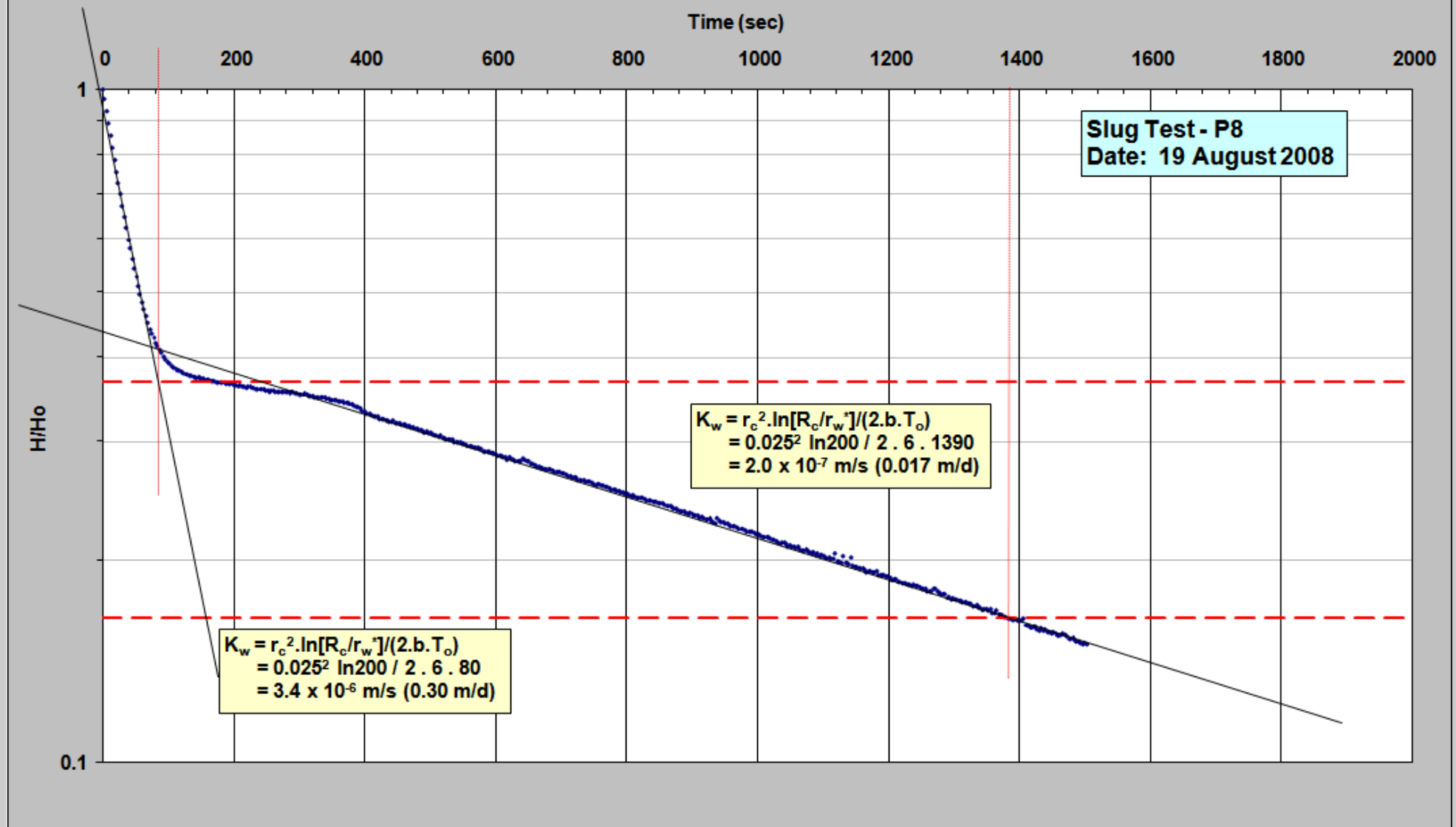


SLUG TEST - P7

Time (sec)

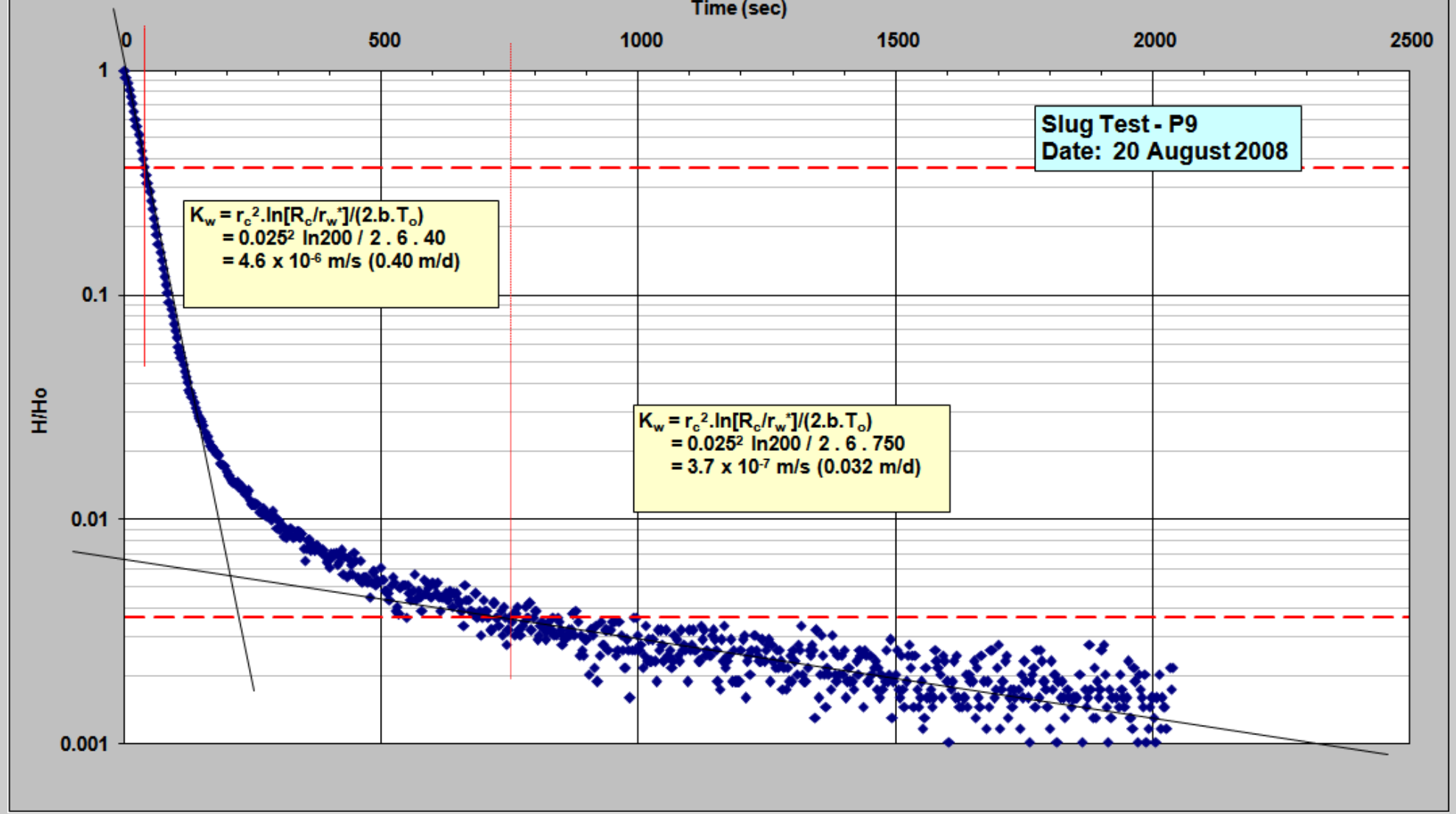


SLUG TEST - P8

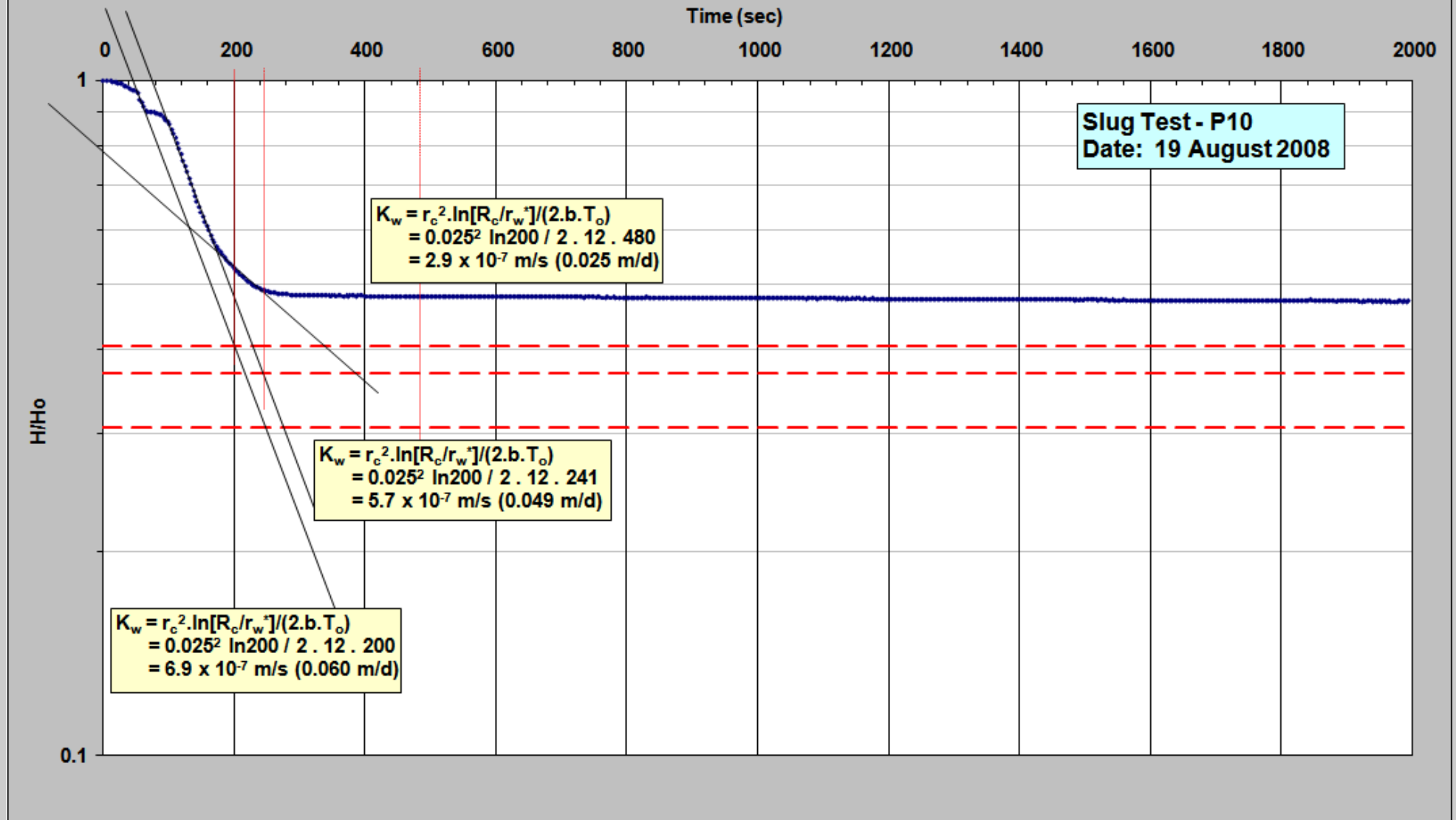


SLUG TEST - P9

Time (sec)



SLUG TEST - P10



SLUG TEST - P11

Time (sec)

0 5000 10000 15000 20000 25000 30000 35000 40000 45000

Slug Test - P11
Date: 19 August 2008

H/Ho

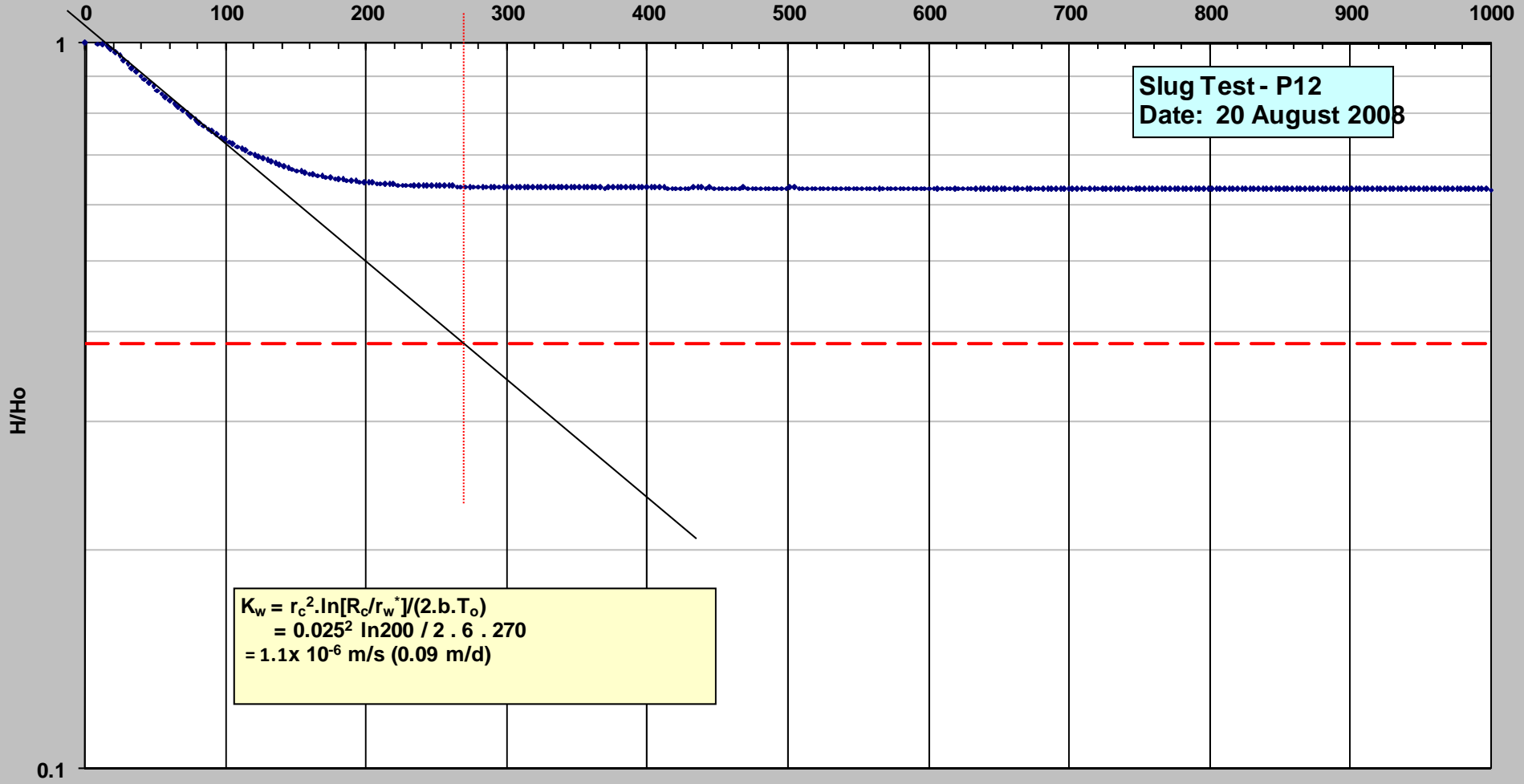
$$\begin{aligned} K_w &= r_c^2 \cdot \ln[R_c/r_w] / (2 \cdot b \cdot T_o) \\ &= 0.025^2 \ln 200 / 2 \cdot 6 \cdot 43000 \\ &= 6.0 \times 10^{-7} \text{ m/s } (5.5 \times 10^{-4} \text{ m/d}) \end{aligned}$$

0.1

SLUG TEST - P12

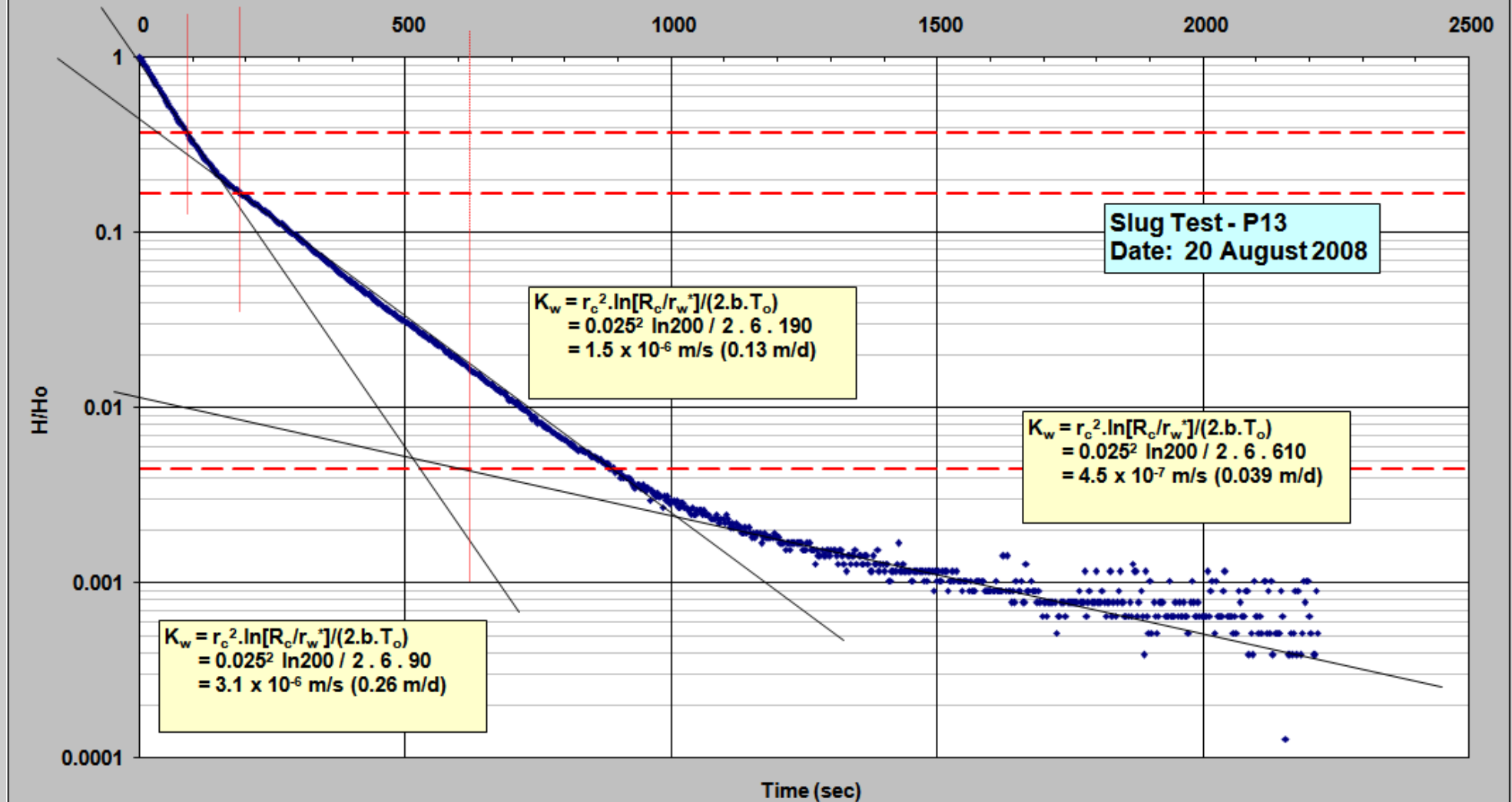
Time (sec)

Slug Test - P12
Date: 20 August 2008

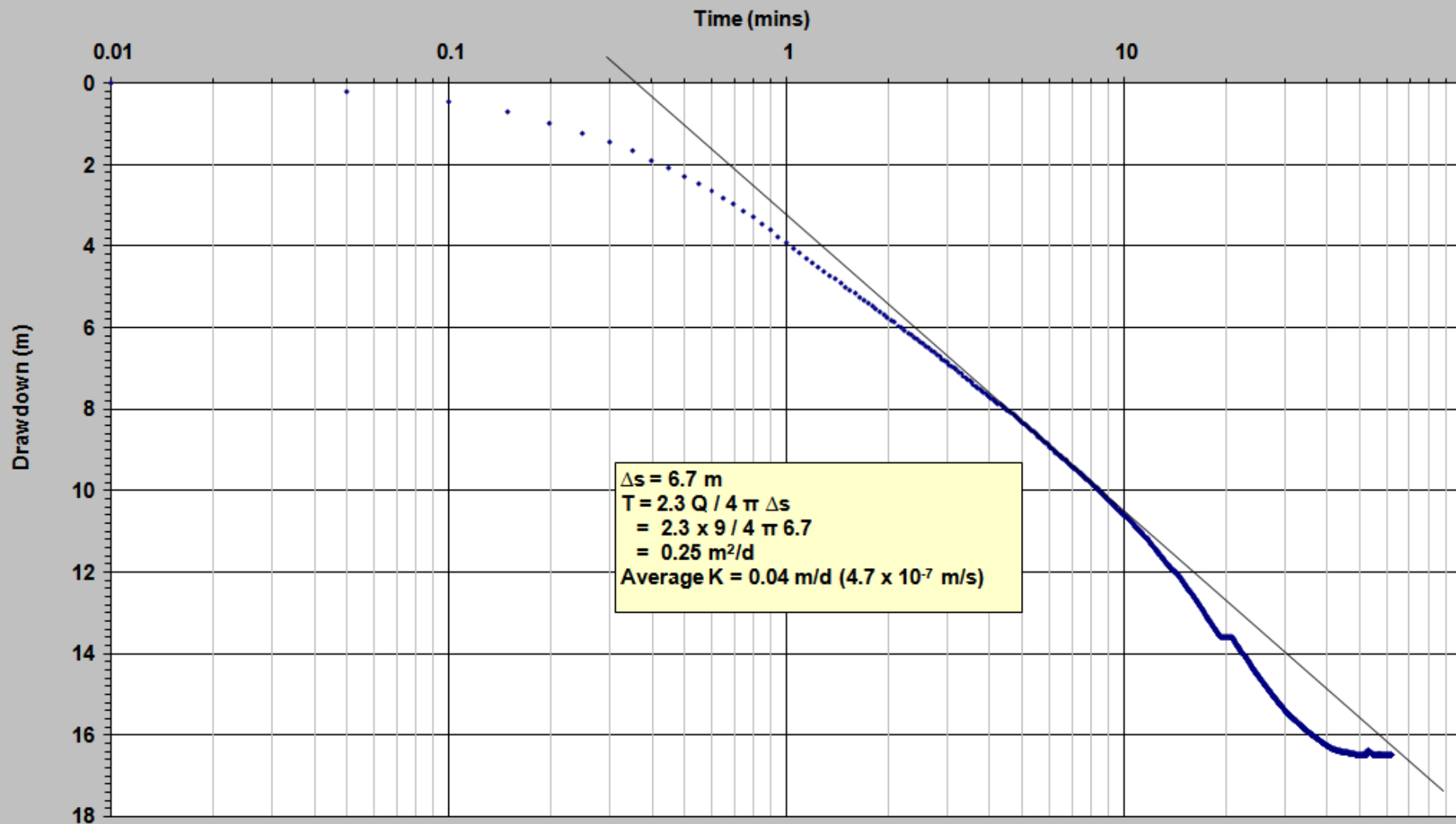


$$\begin{aligned}
 K_w &= r_c^2 \cdot \ln[R_c/r_w^*] / (2 \cdot b \cdot T_o) \\
 &= 0.025^2 \ln 200 / 2 \cdot 6 \cdot 270 \\
 &= 1.1 \times 10^{-6} \text{ m/s (0.09 m/d)}
 \end{aligned}$$

SLUG TEST - P13

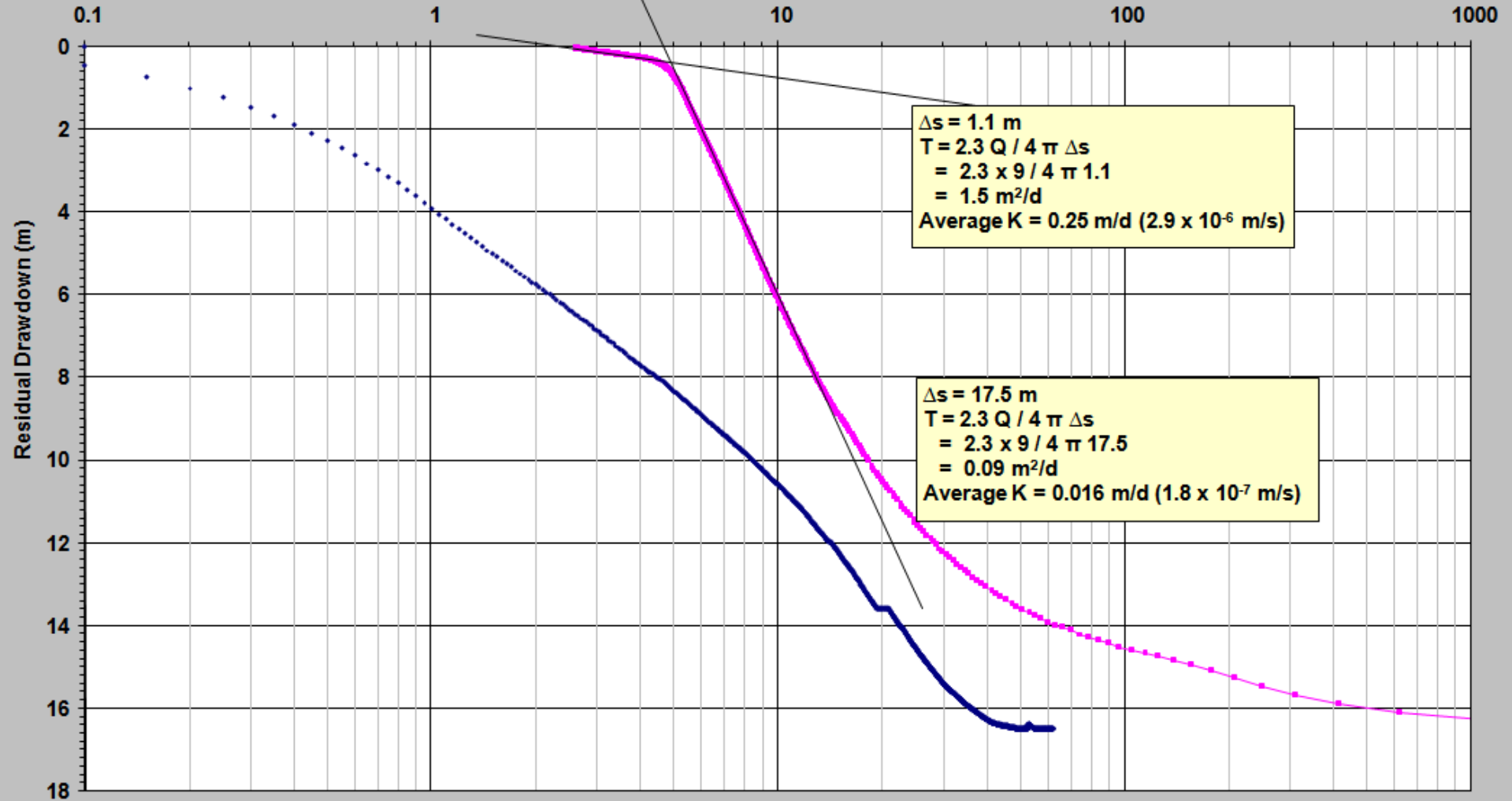


CONSTANT RATE TEST - Drawdown-P13

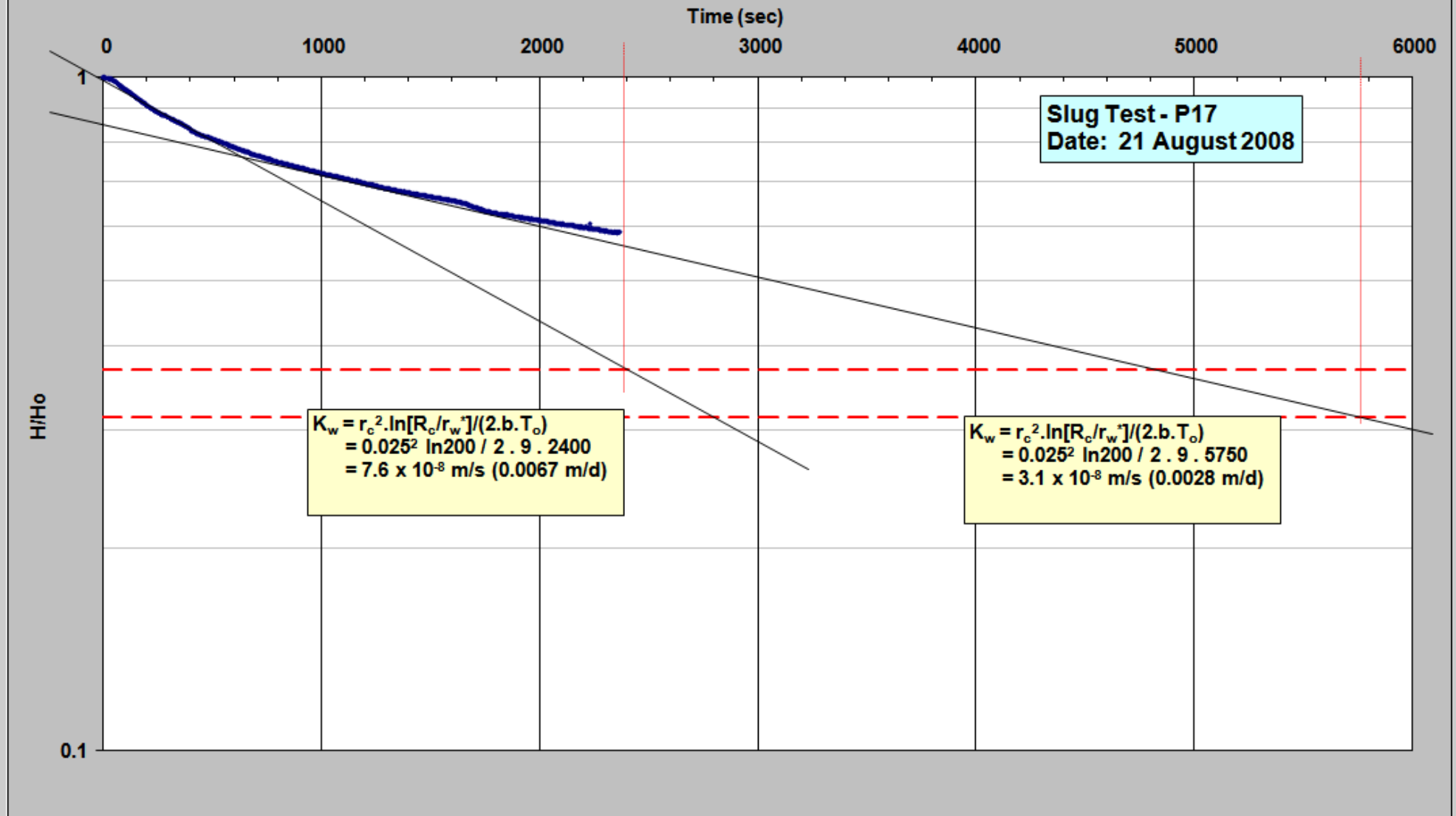


CONSTANT RATE TEST - P13

Time (min) or t/t'



SLUG TEST - P17



SLUG TEST - P18

Time (sec)

0 500 1000 1500 2000 2500 3000 3500 4000

Slug Test - P18
Date: 22 August 2008

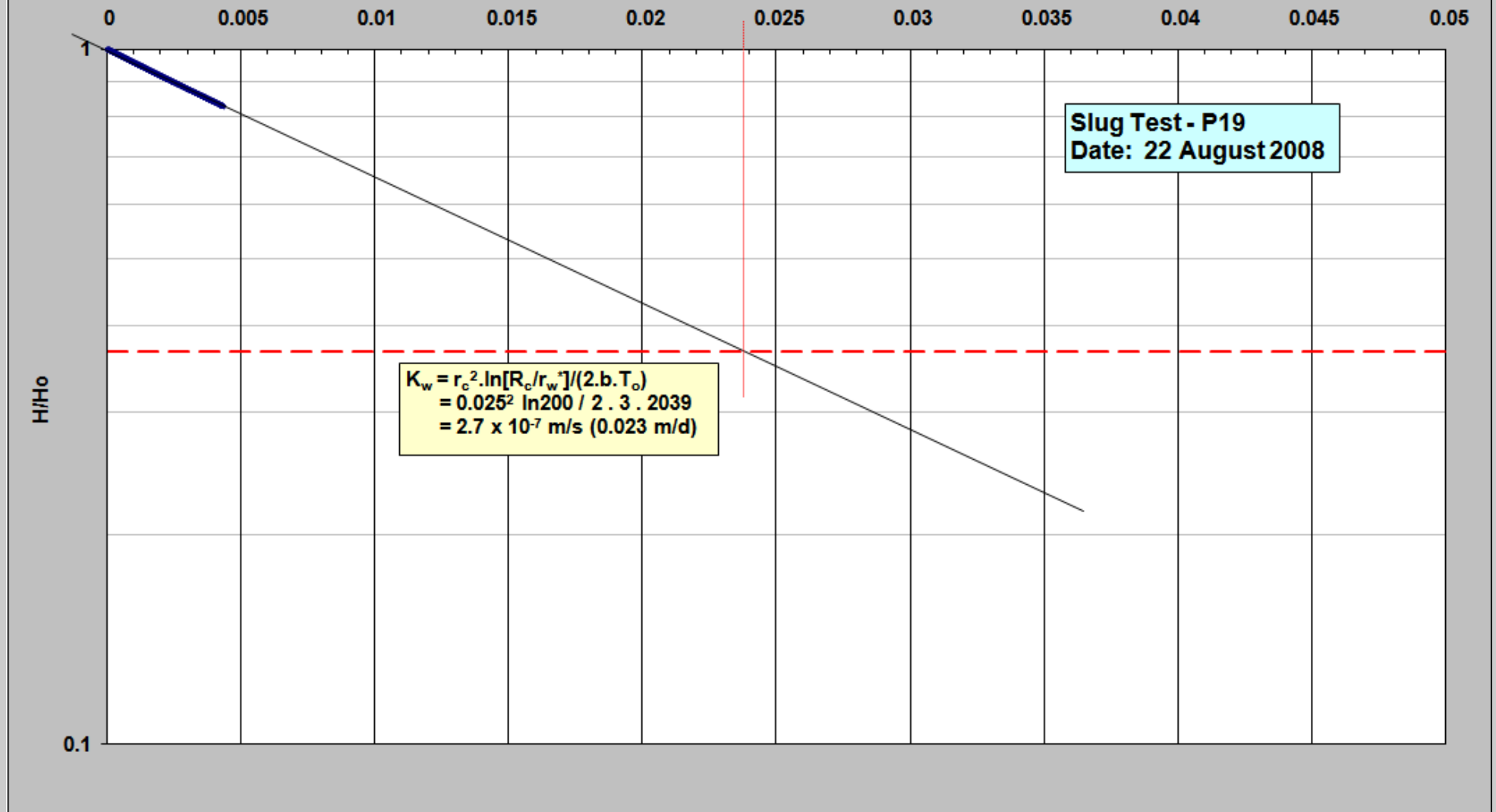
$$\begin{aligned} K_w &= r_c^2 \cdot \ln[R_c/r_w] / (2 \cdot b \cdot T_o) \\ &= 0.025^2 \ln 200 / 2 \cdot 3 \cdot 3655 \\ &= 1.7 \times 10^{-7} \text{ m/s (0.015 m/d)} \end{aligned}$$

H/Ho

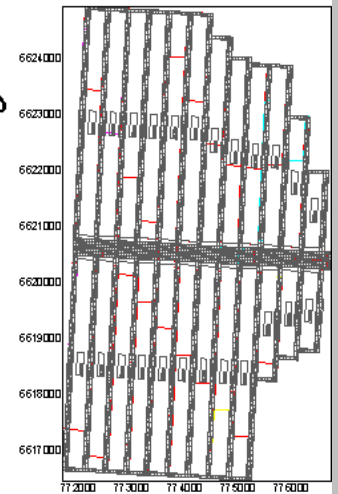
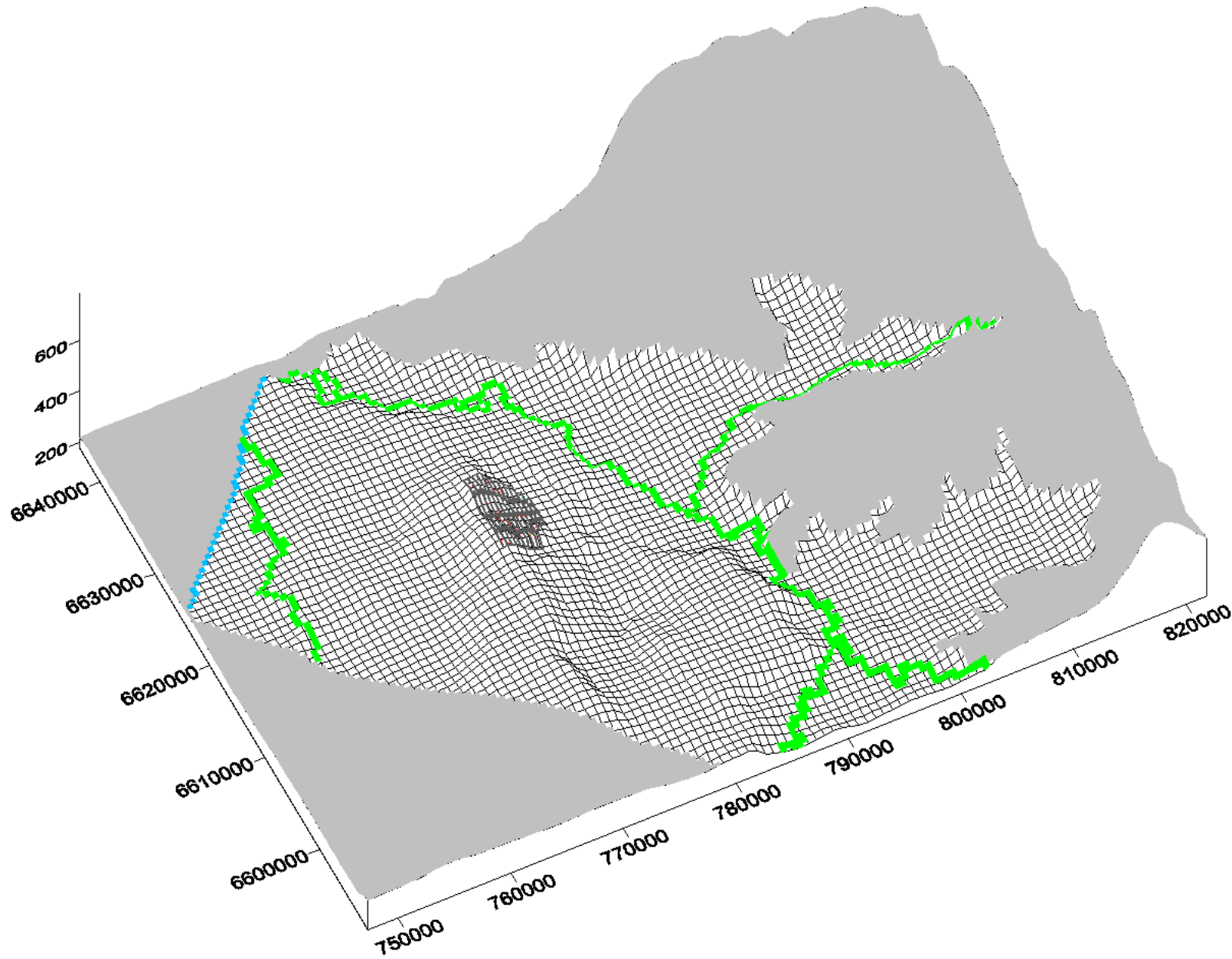
0.1

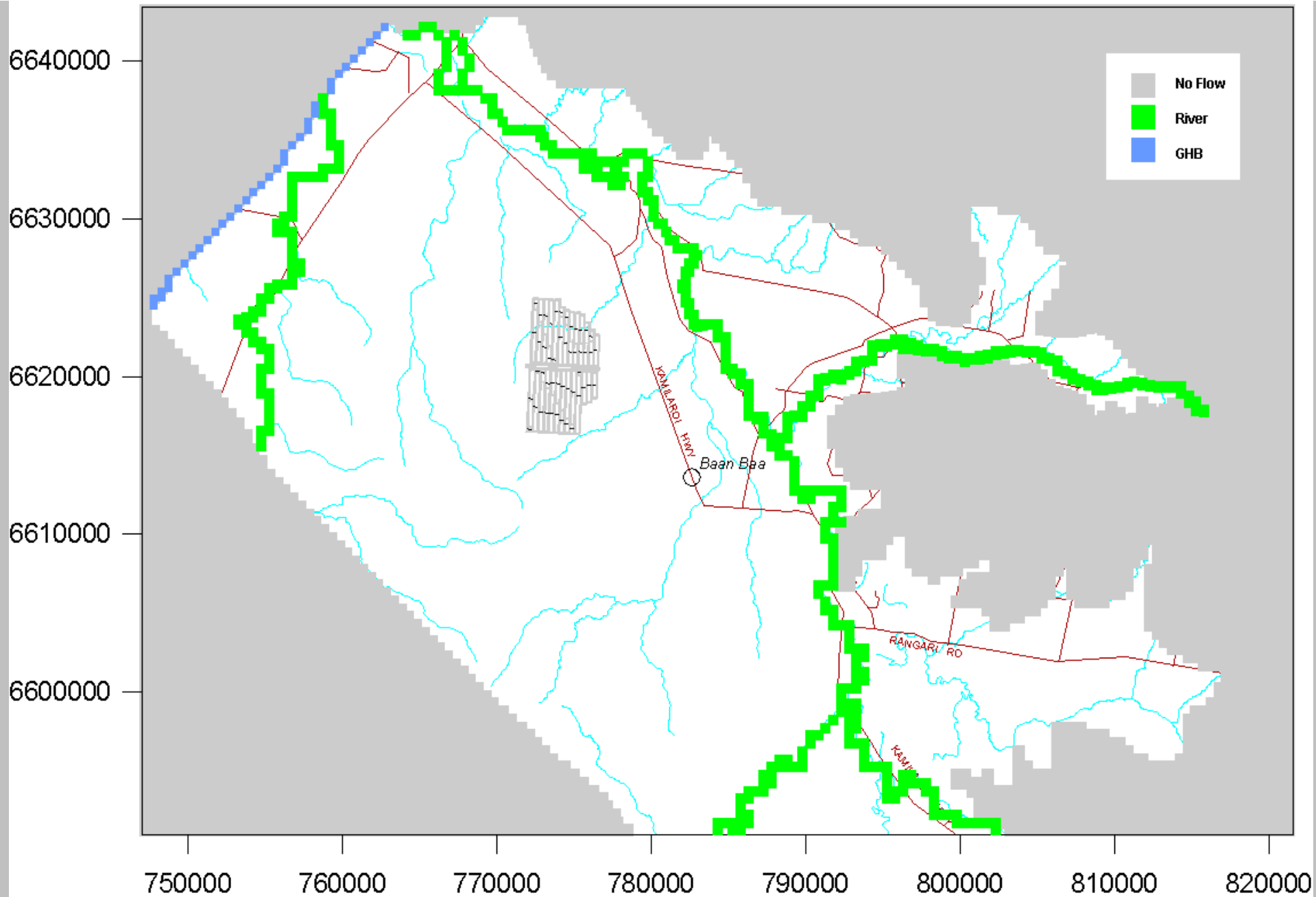
SLUG TEST - P19

Time (10×days)



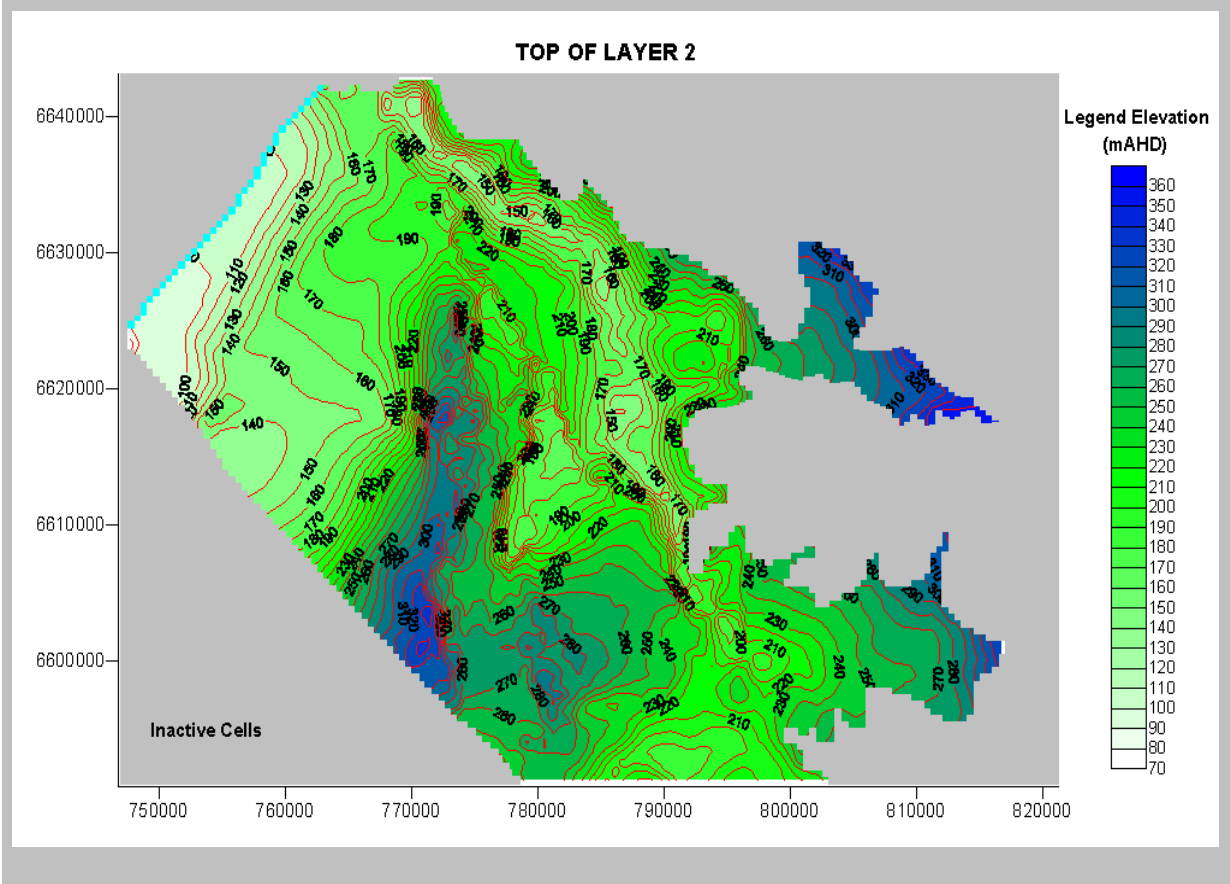
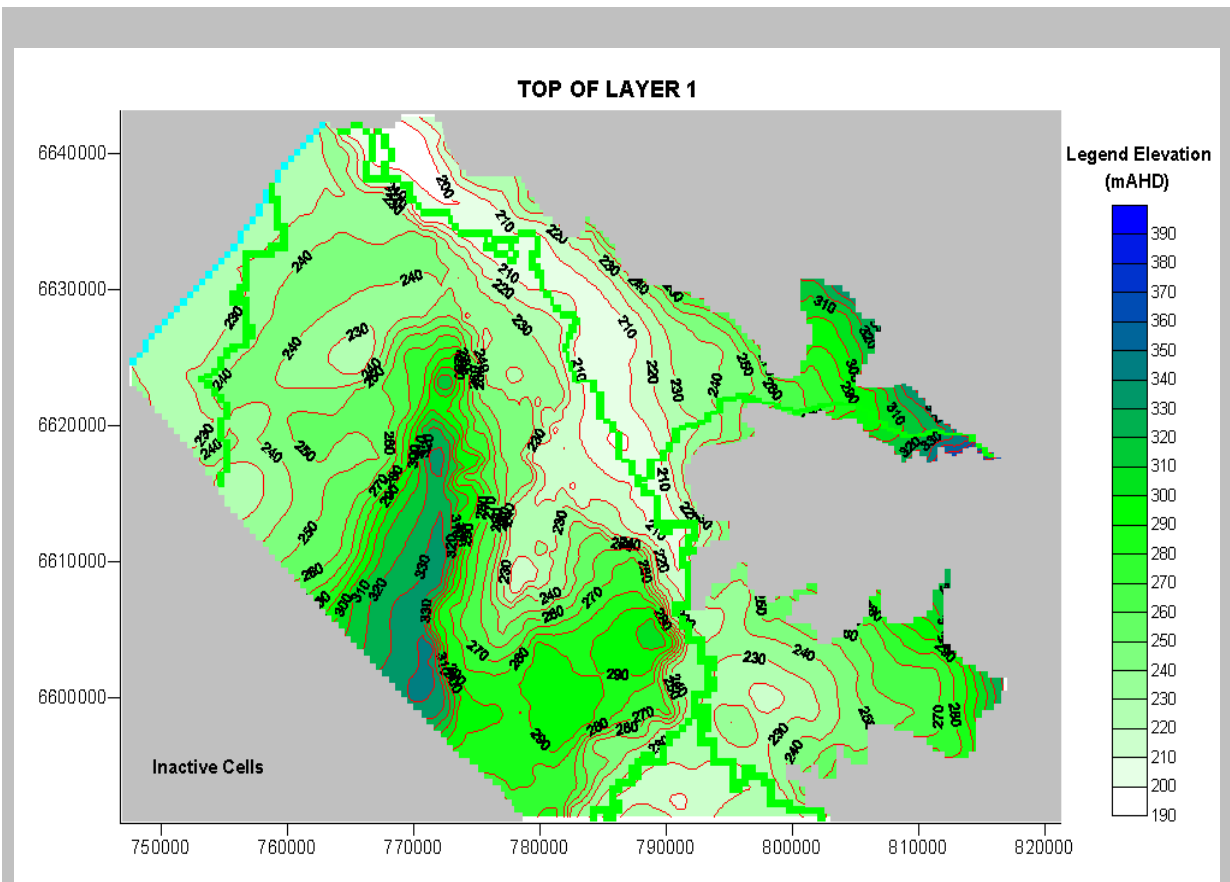
APPENDIX D
MODEL DOMAIN AND LAYER BOUNDARY
CONDITIONS

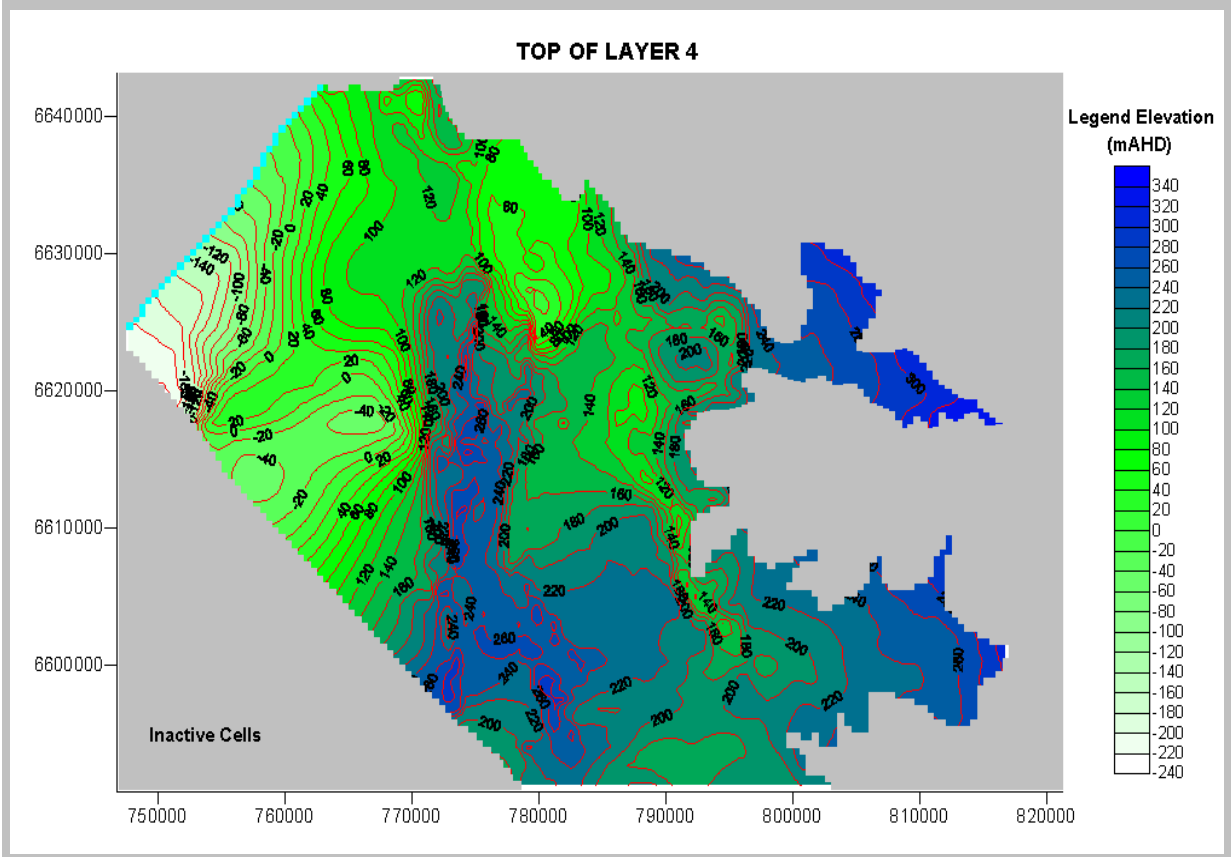
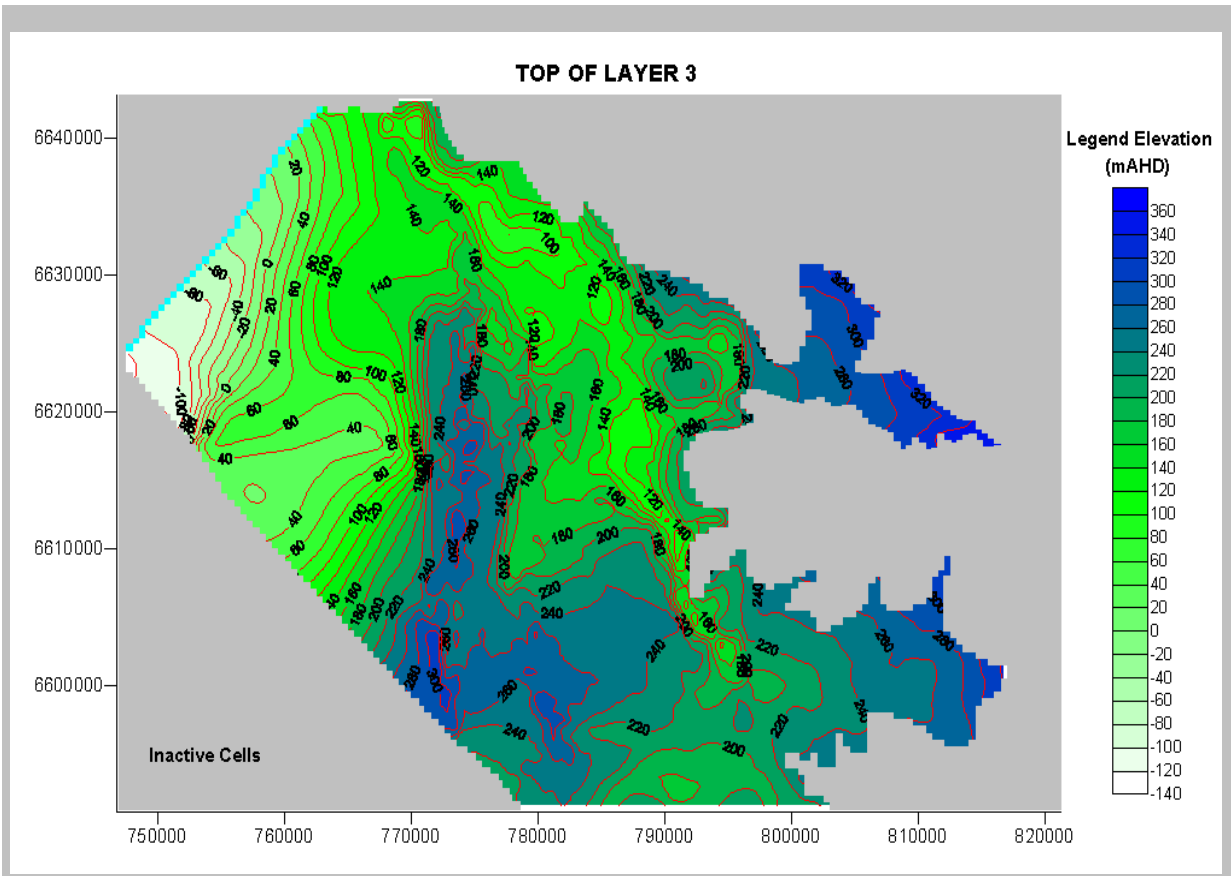




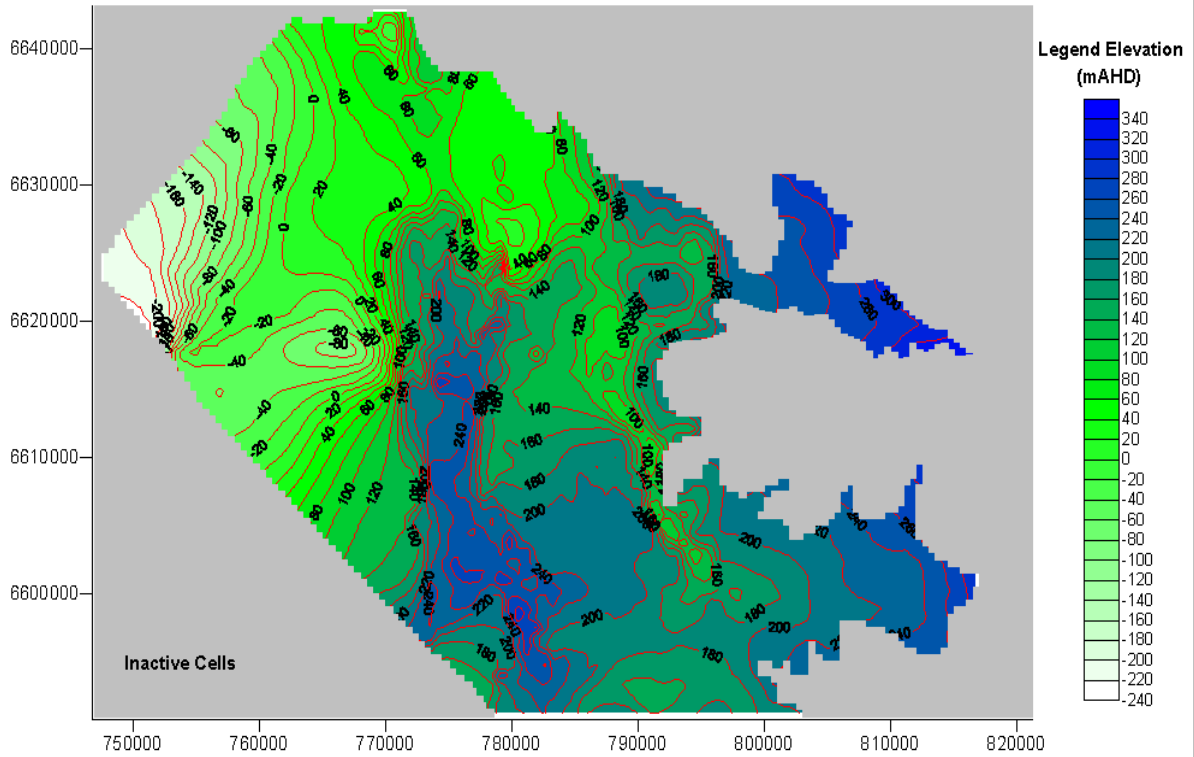
APPENDIX E

NARRABRI MODEL LAYER ELEVATIONS

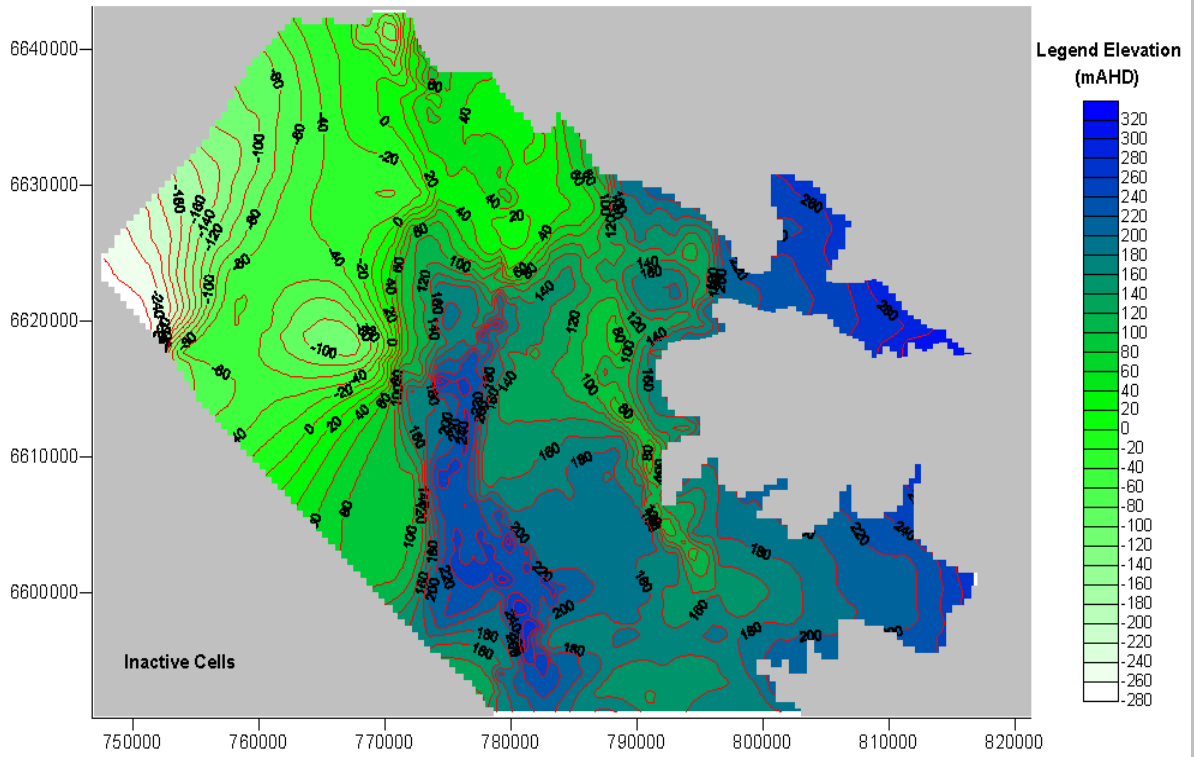


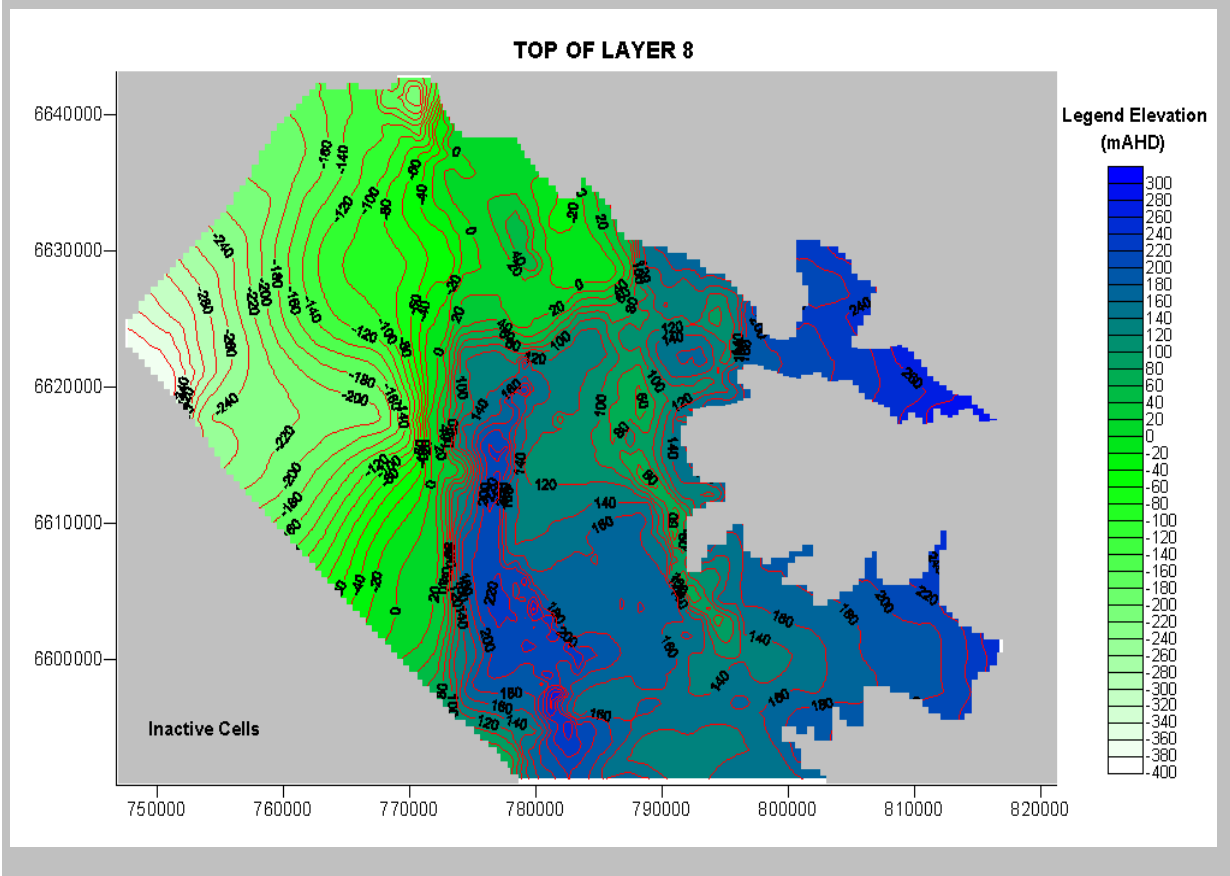
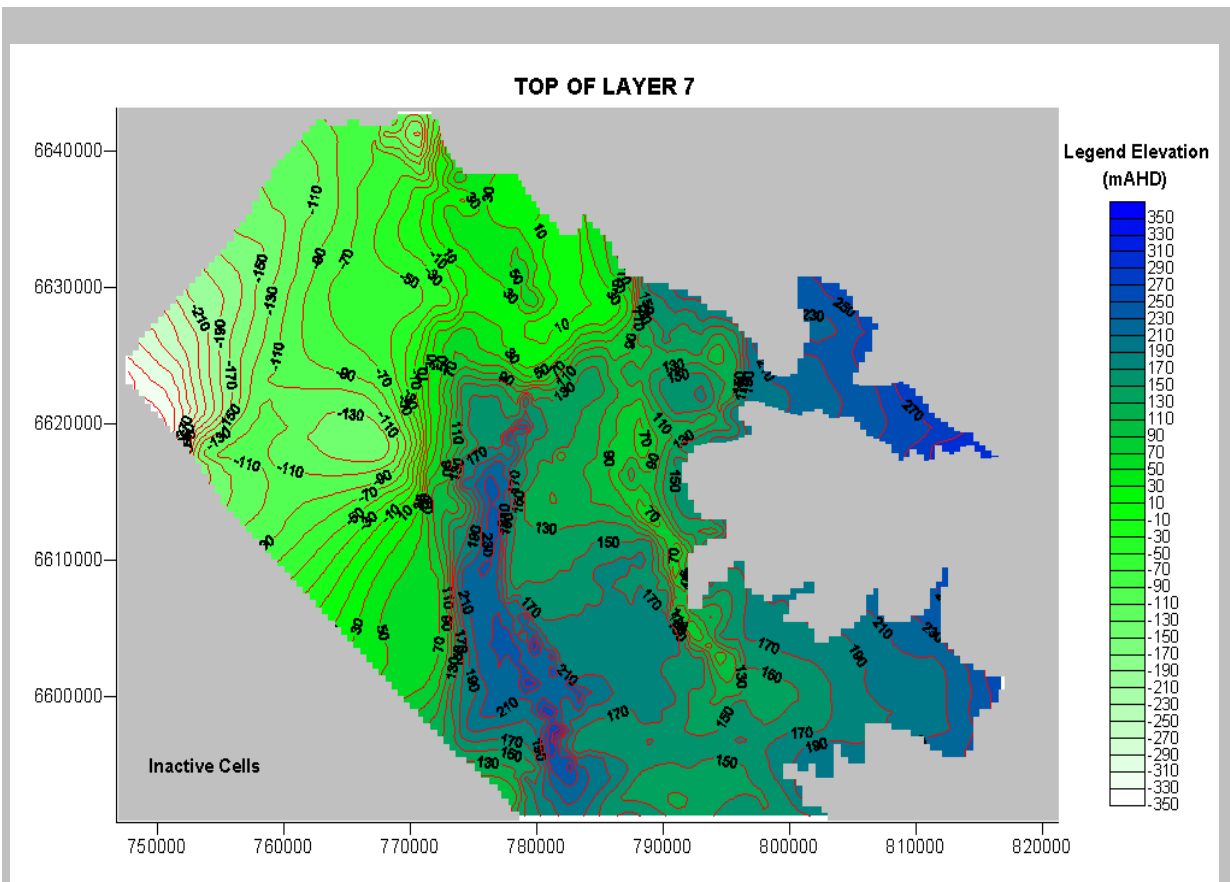


TOP OF LAYER 5

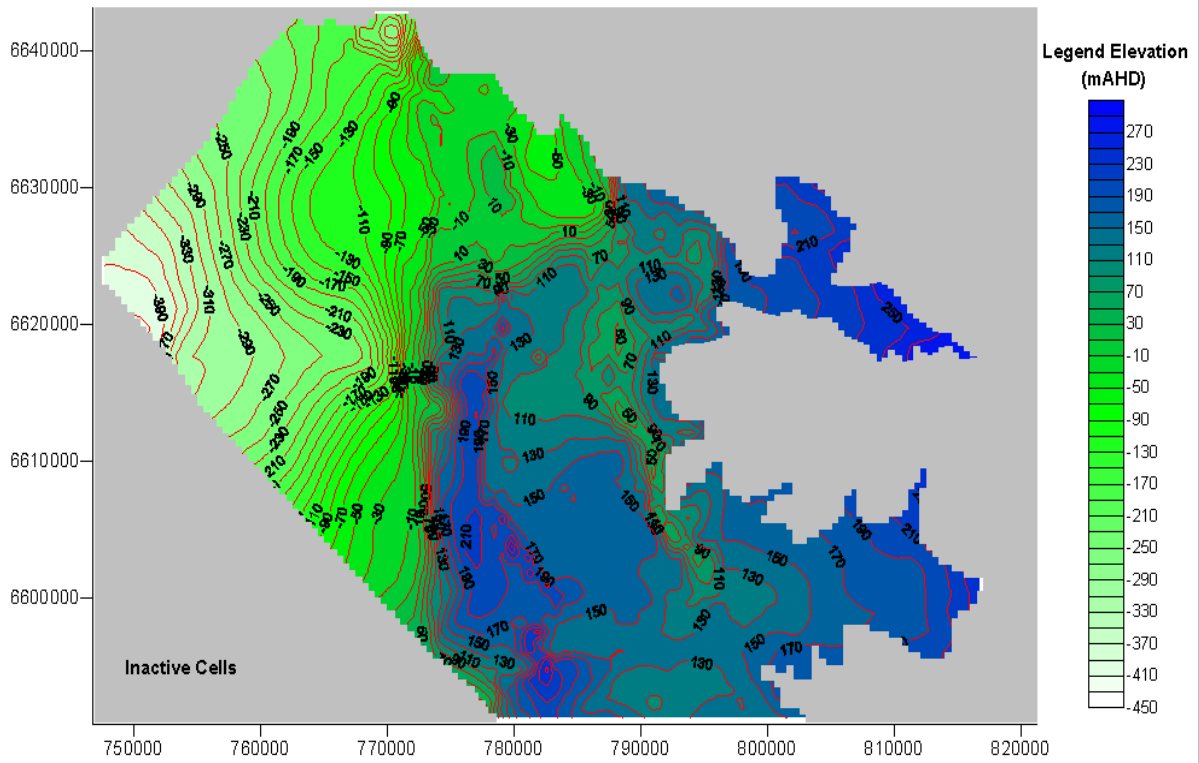


TOP OF LAYER 6

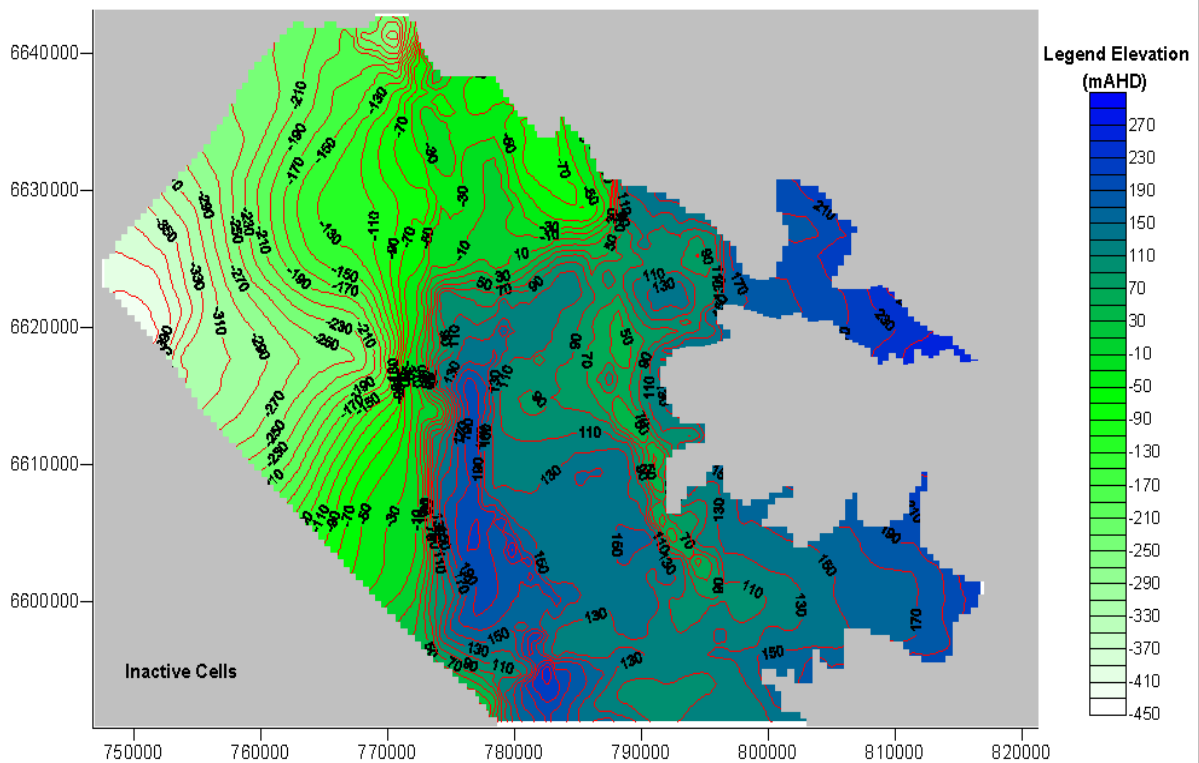


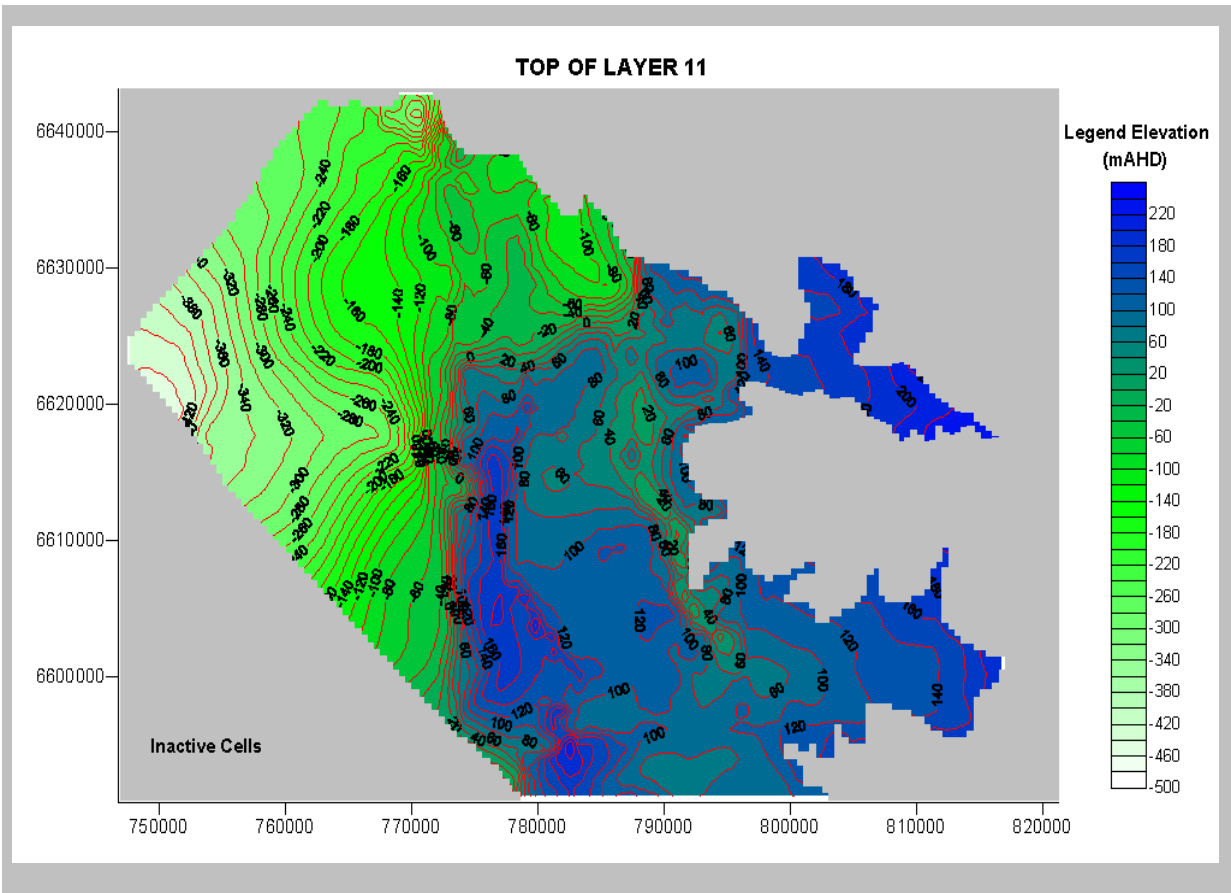


TOP OF LAYER 9



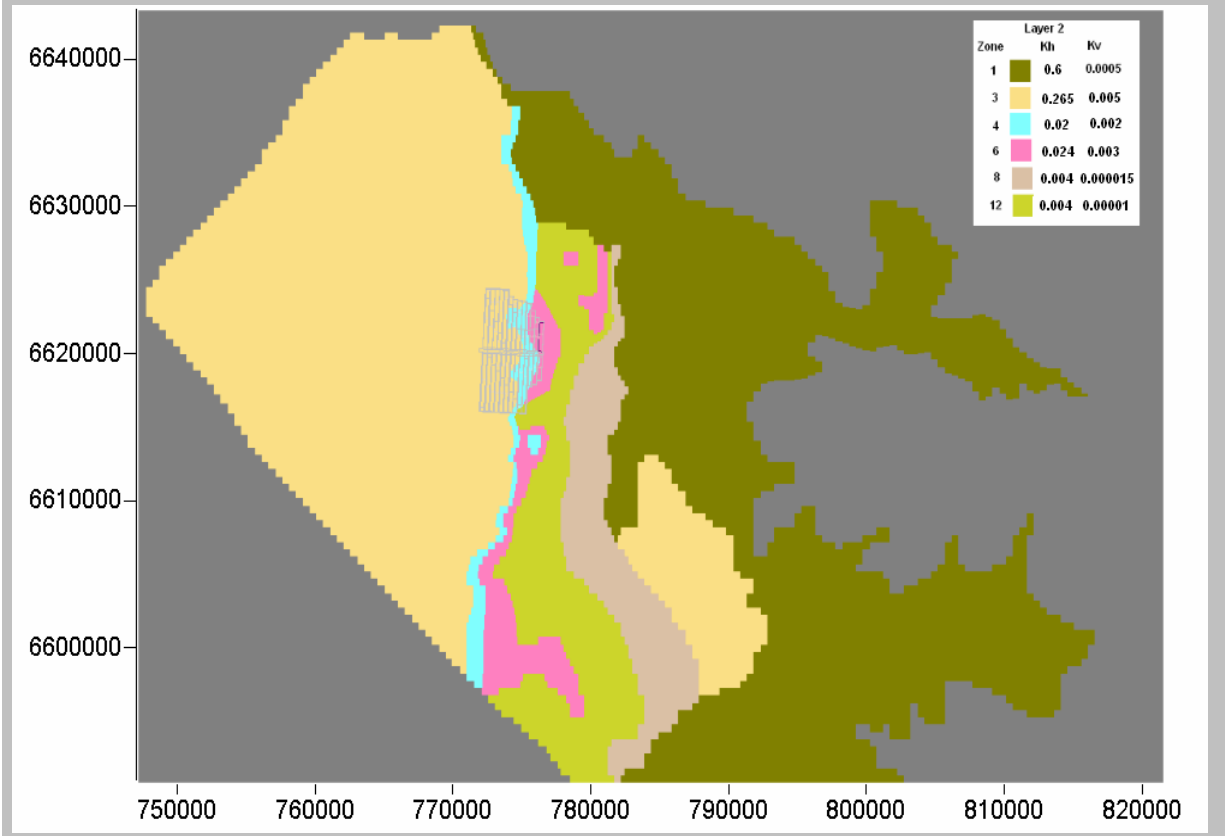
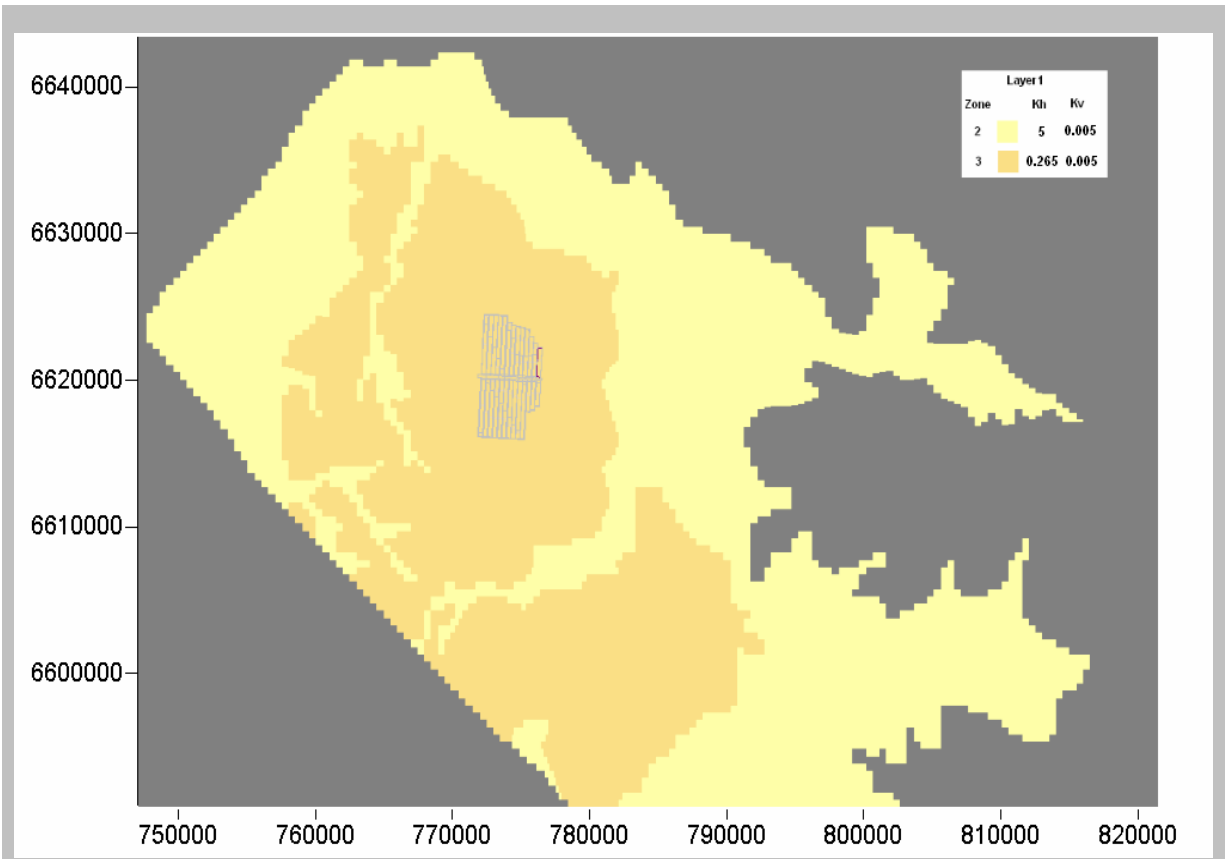
TOP OF LAYER 10

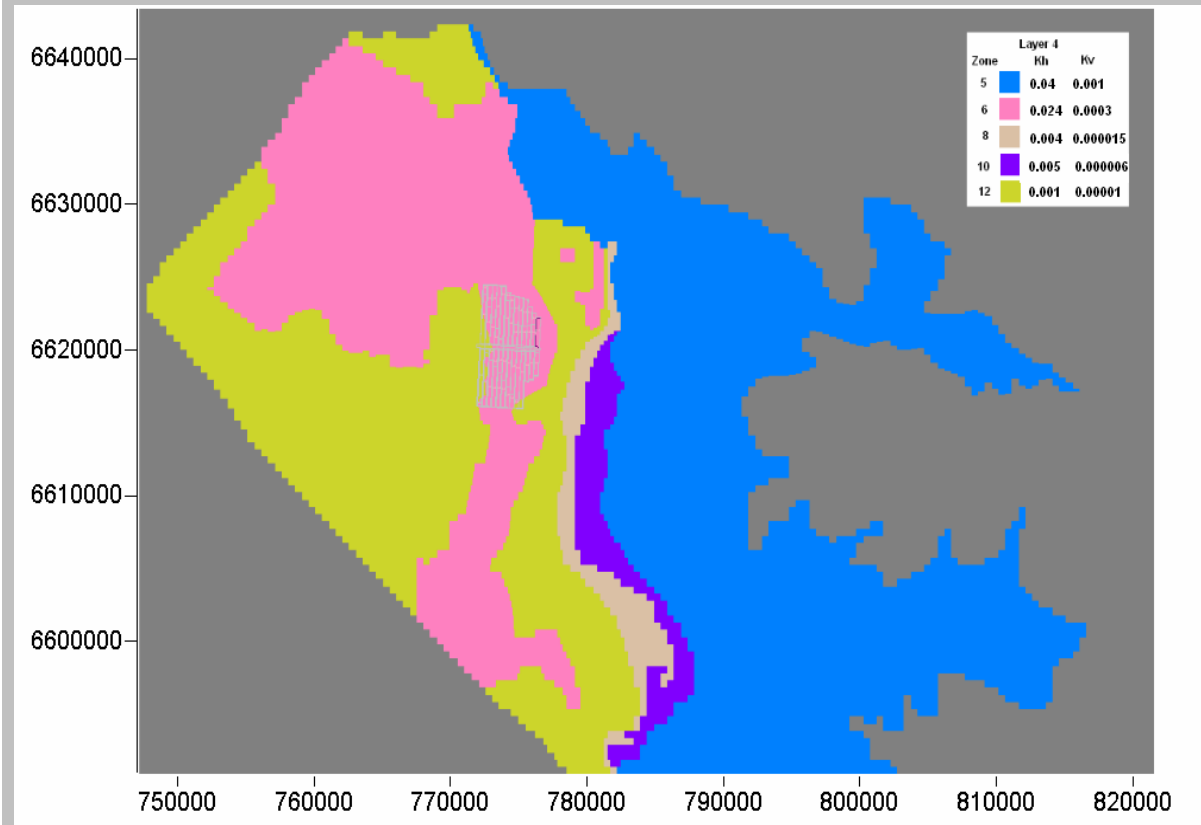
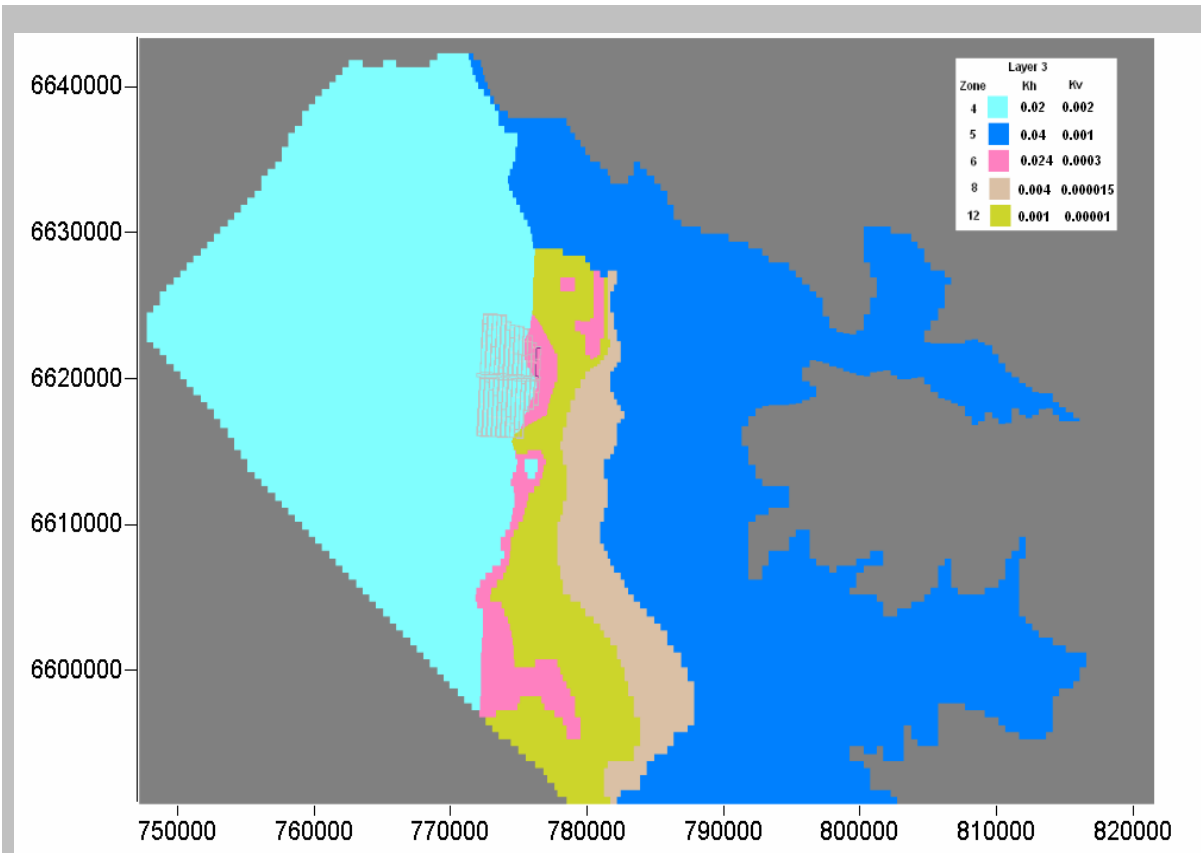


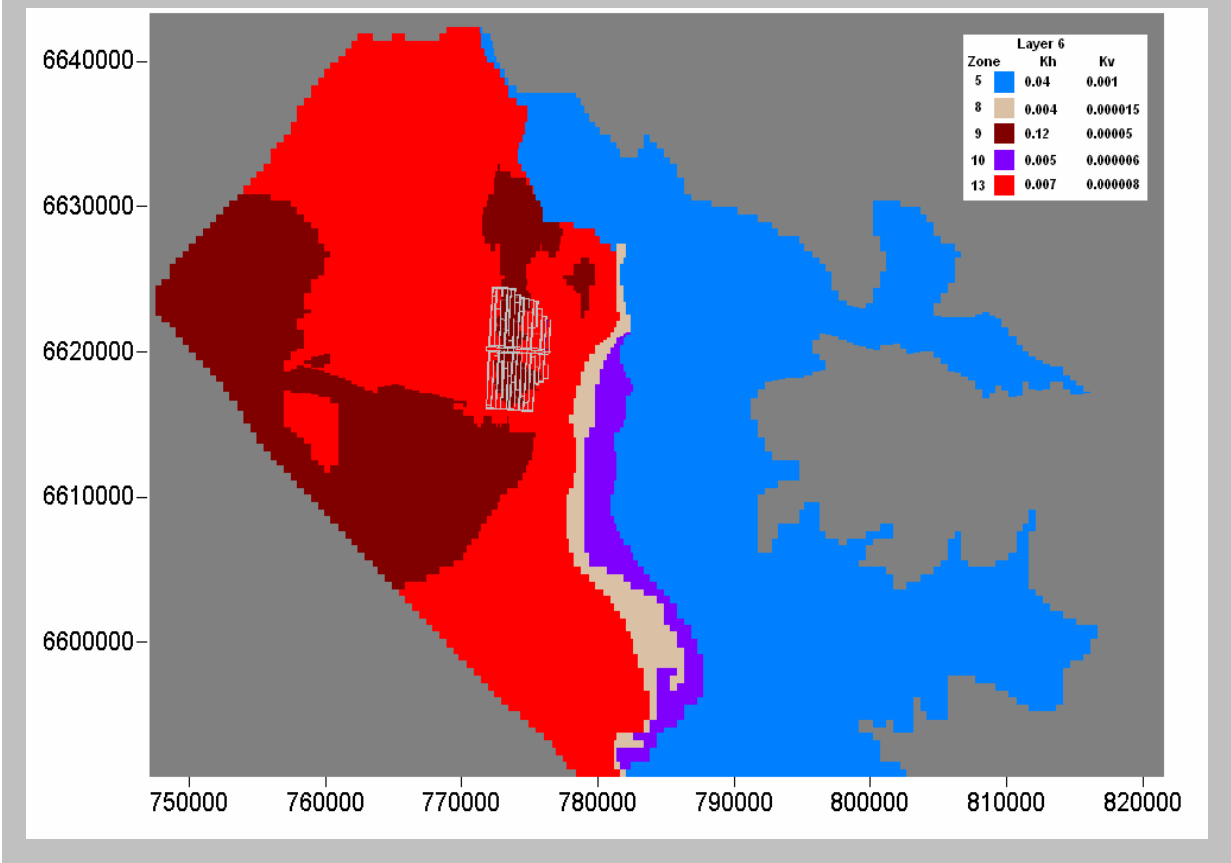
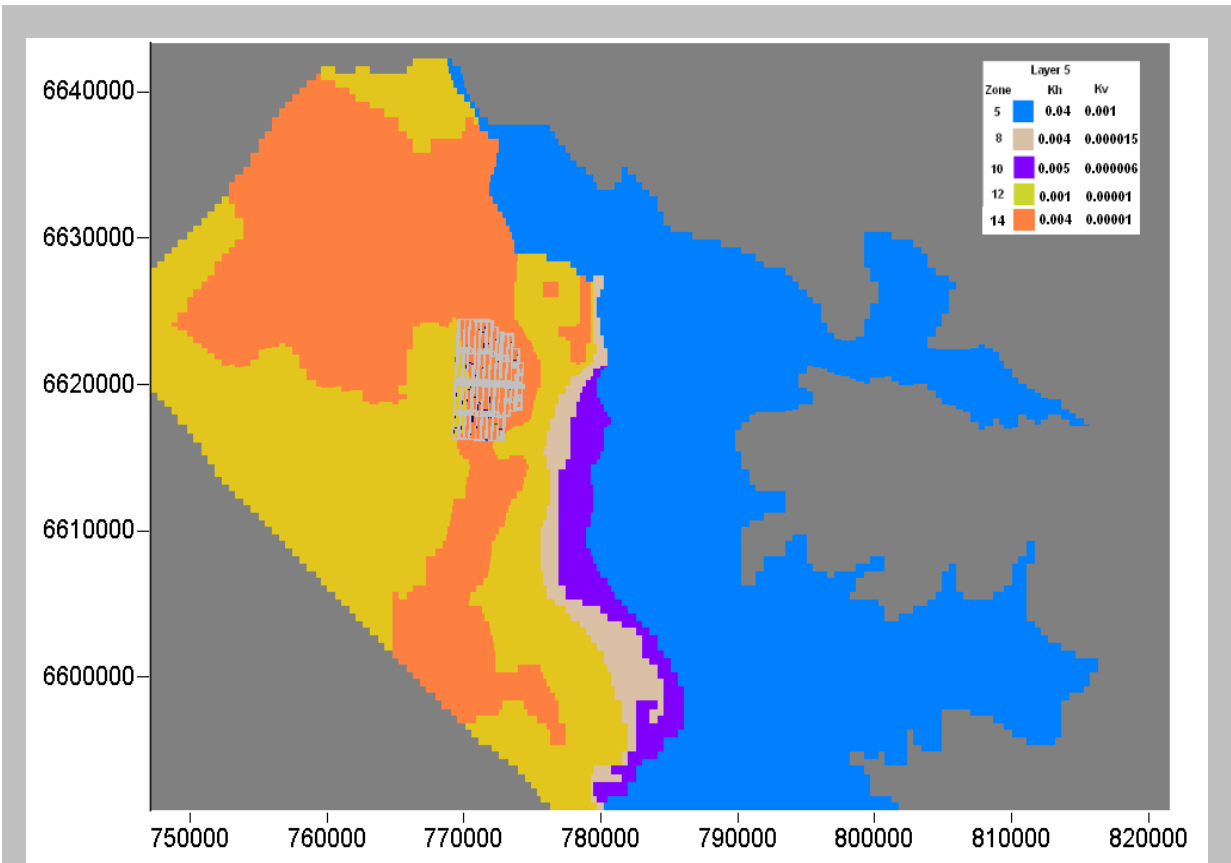


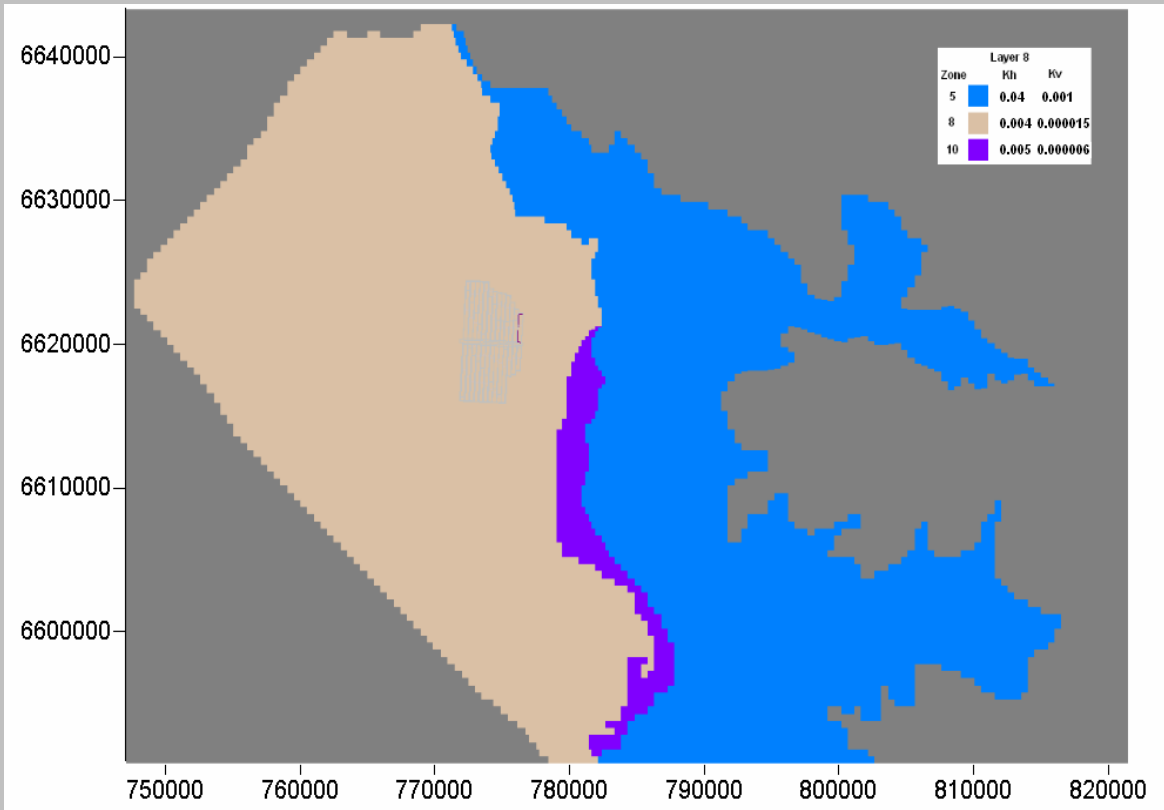
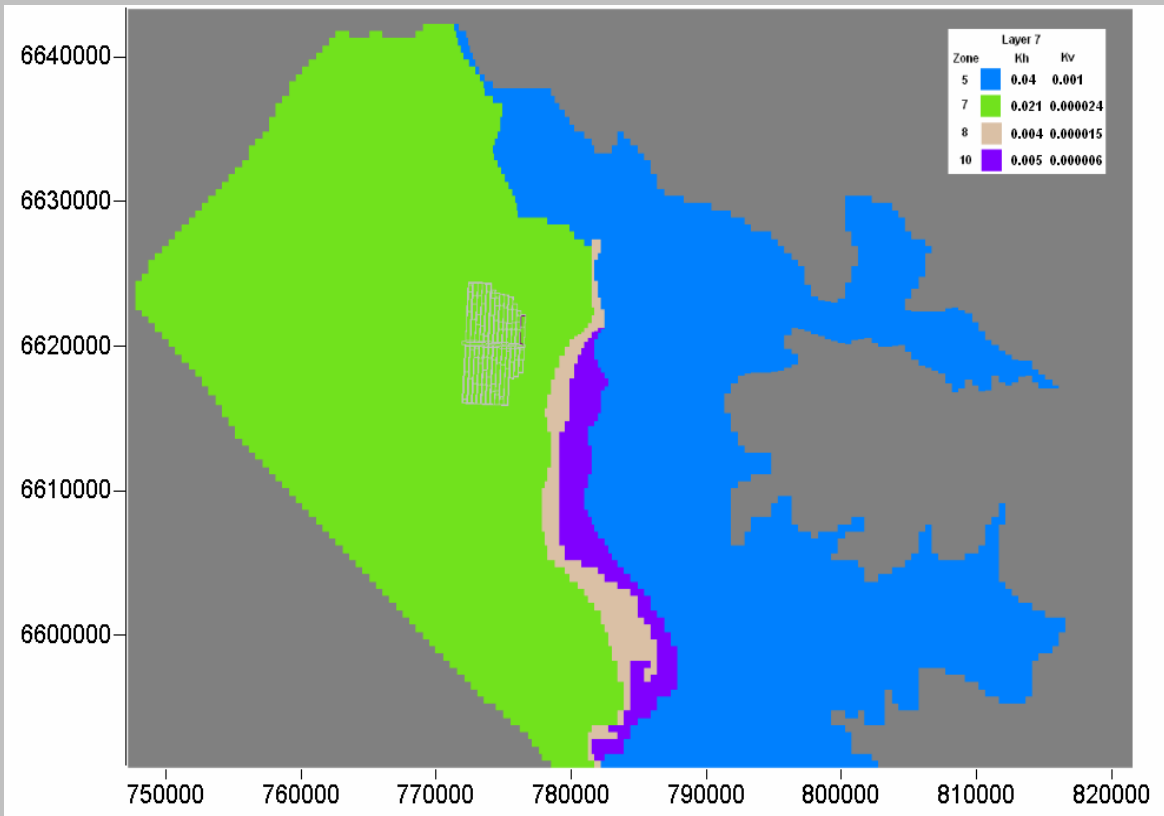
APPENDIX F

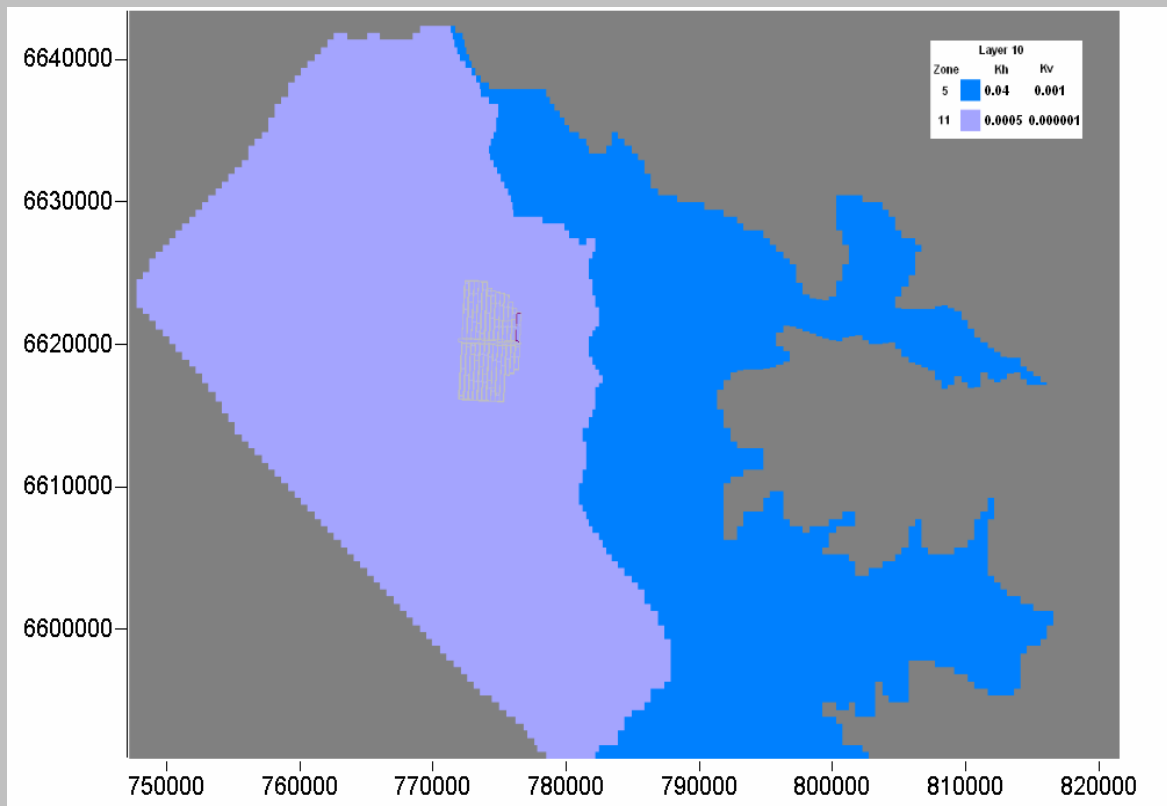
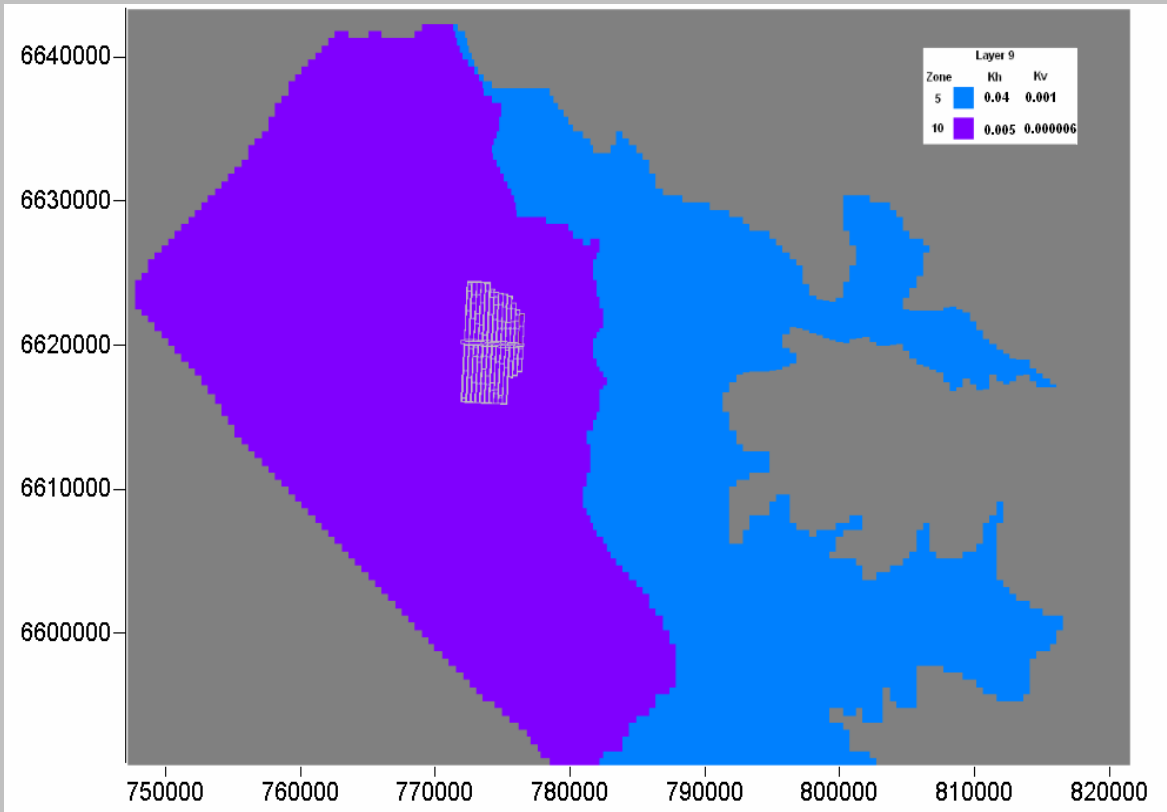
NARRABRI MODEL CALIBRATED PARAMETERS

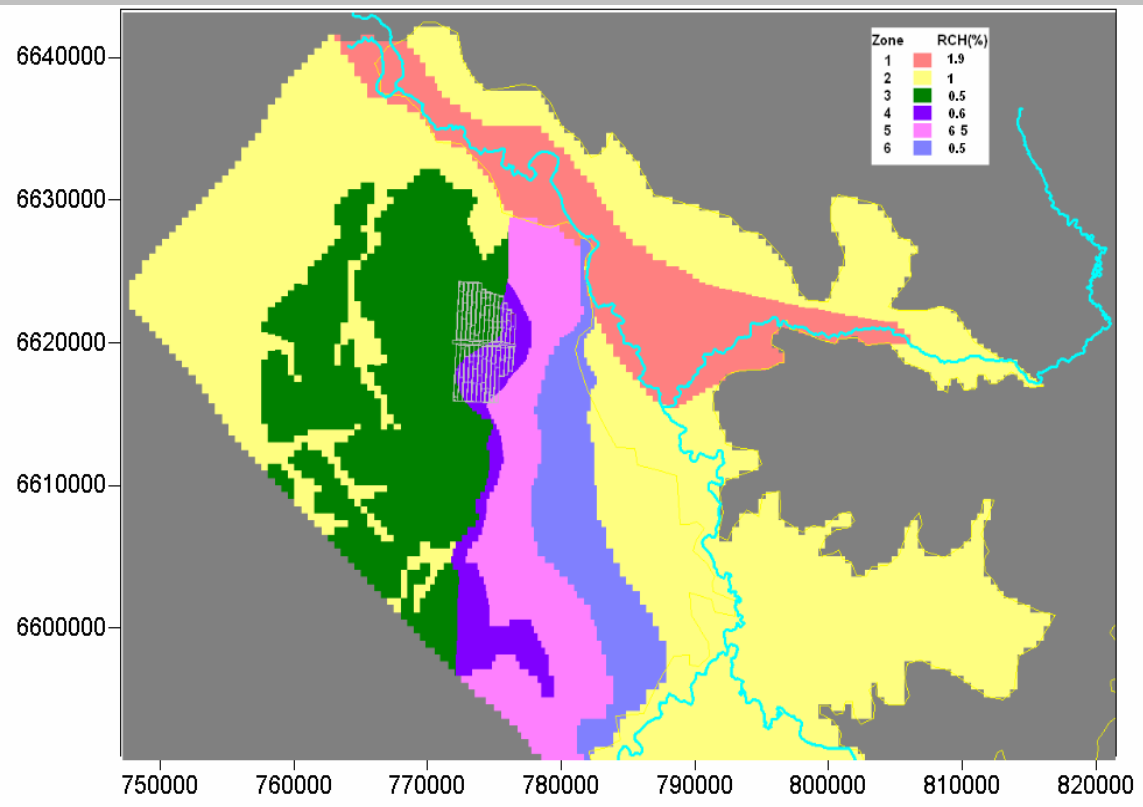
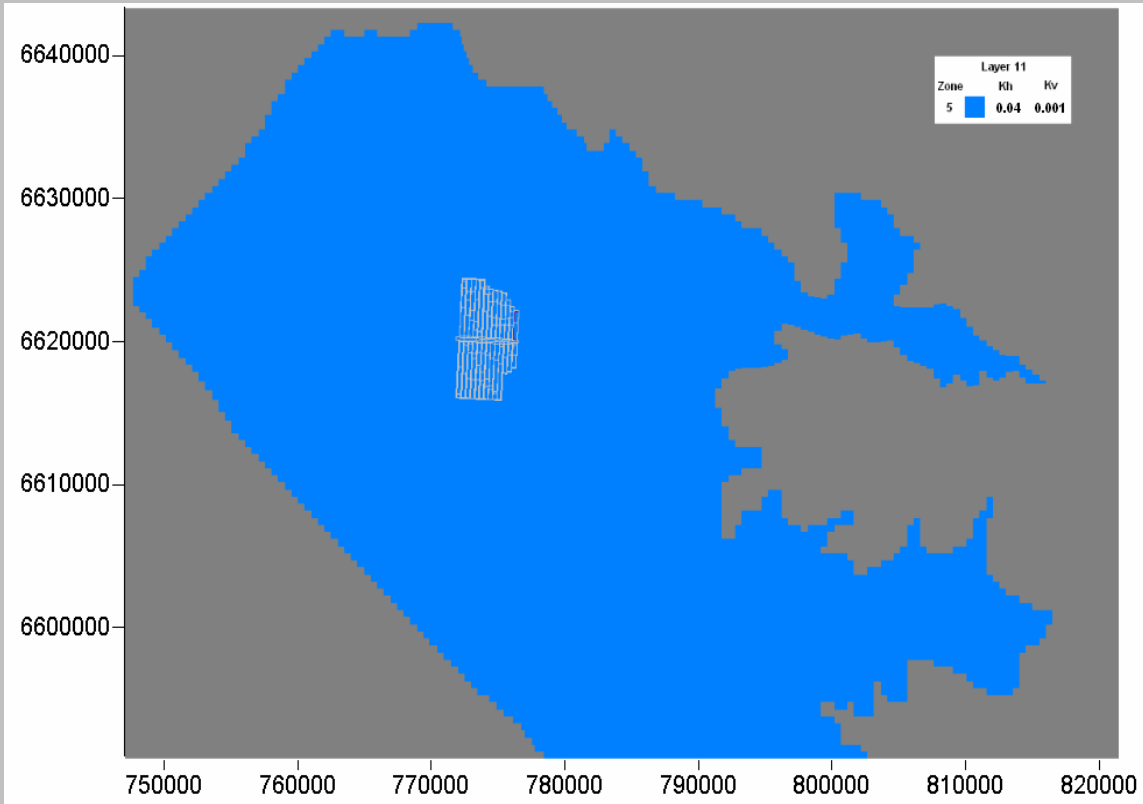








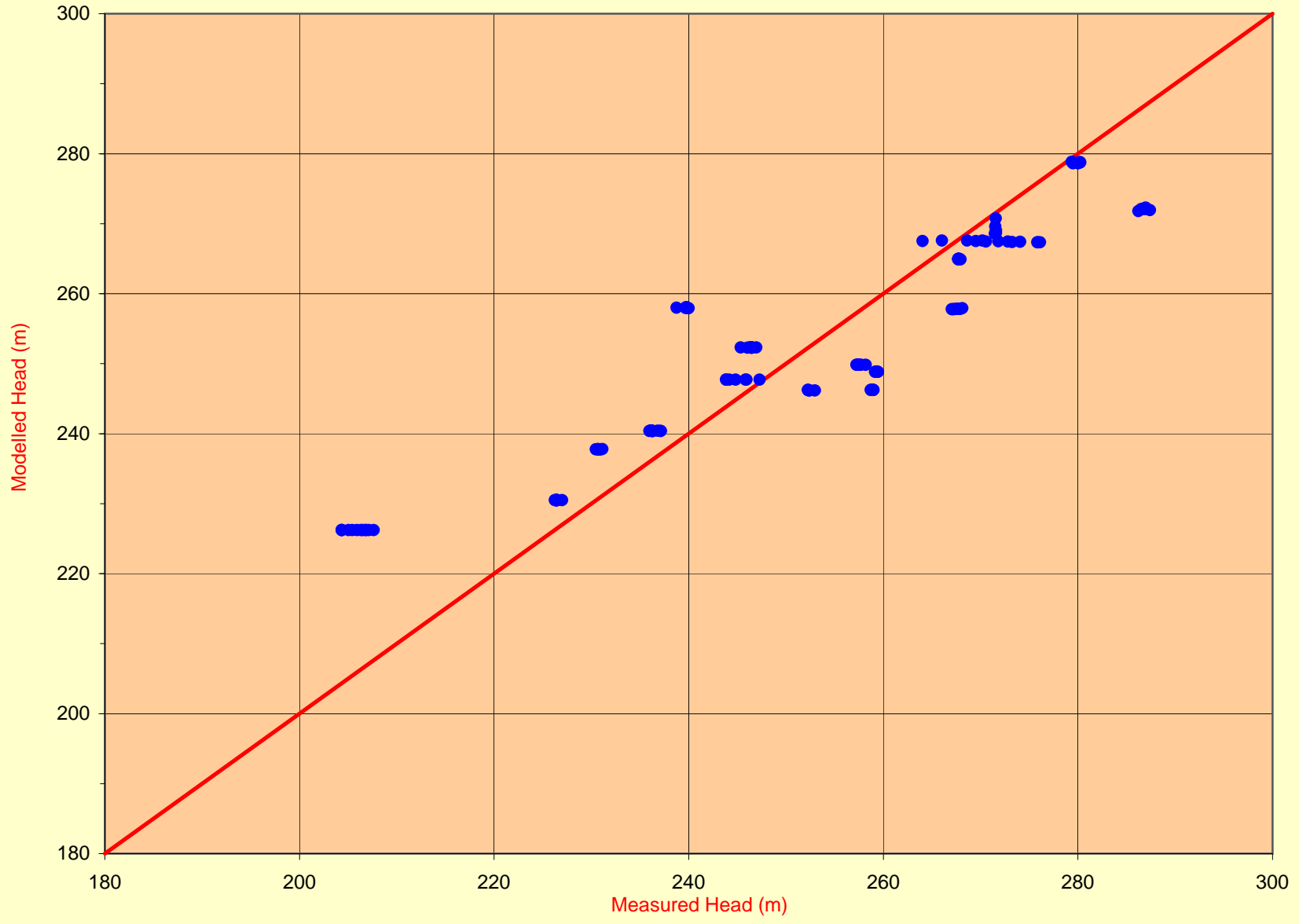




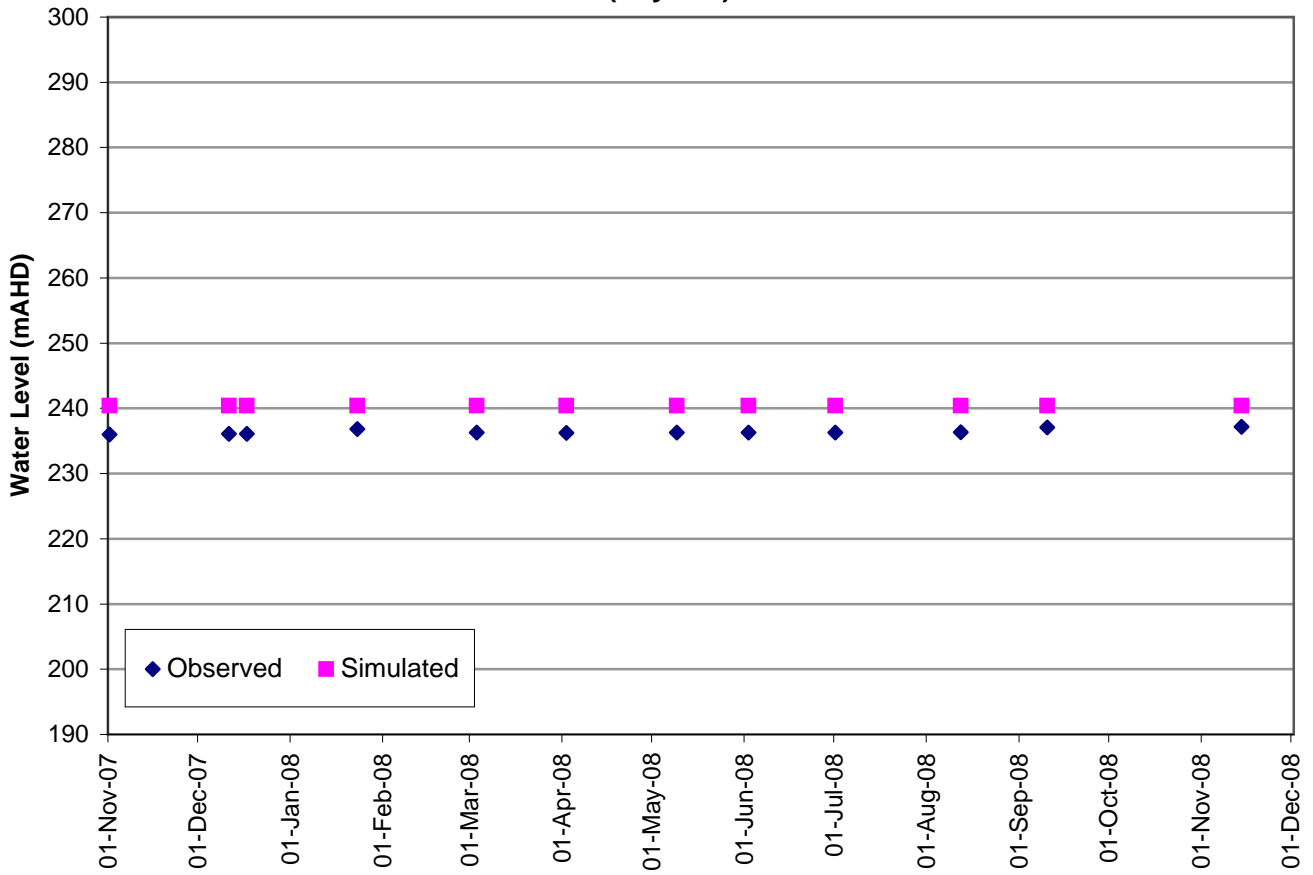
APPENDIX G

NARRABRI MODEL TRANSIENT CALIBRATION

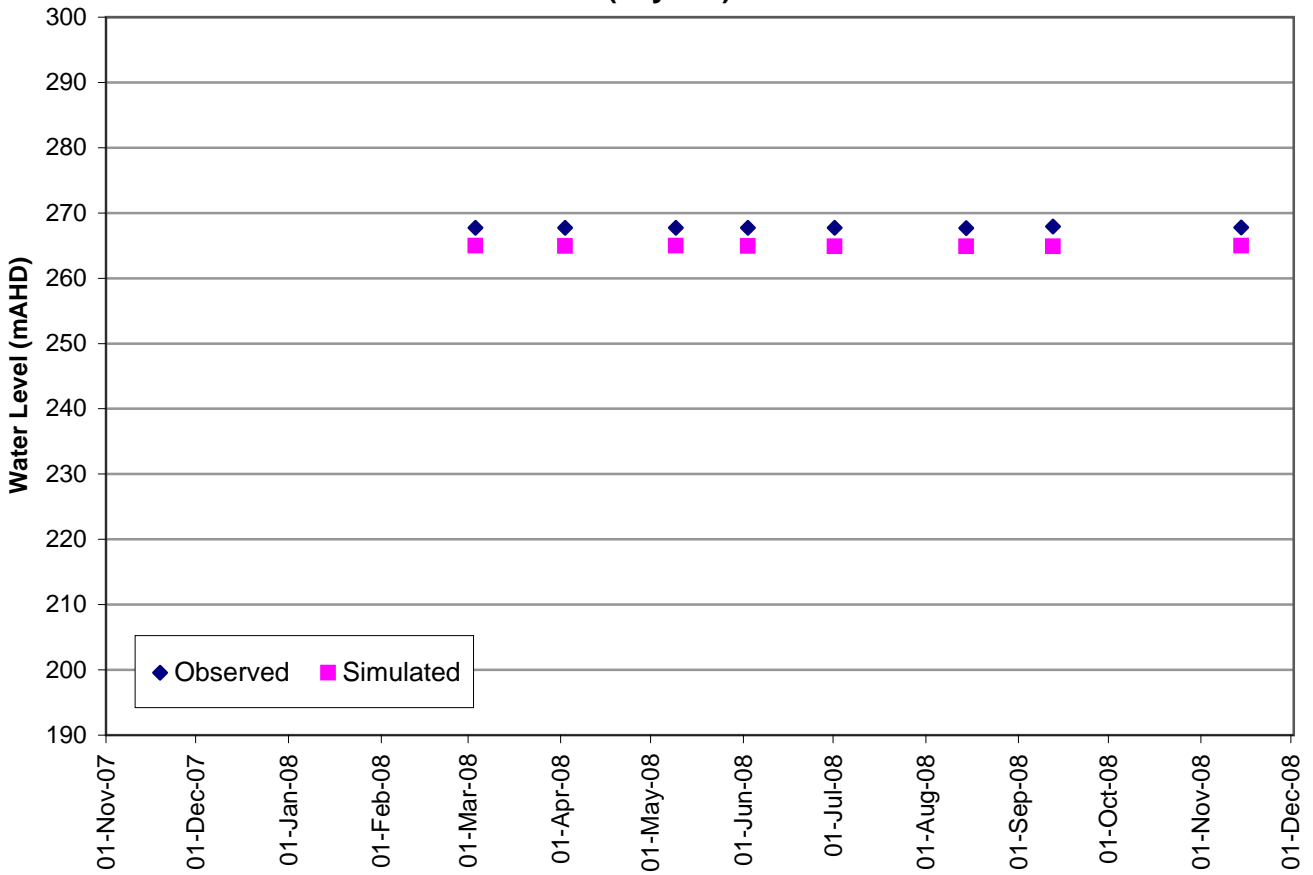
SCATTERGRAM



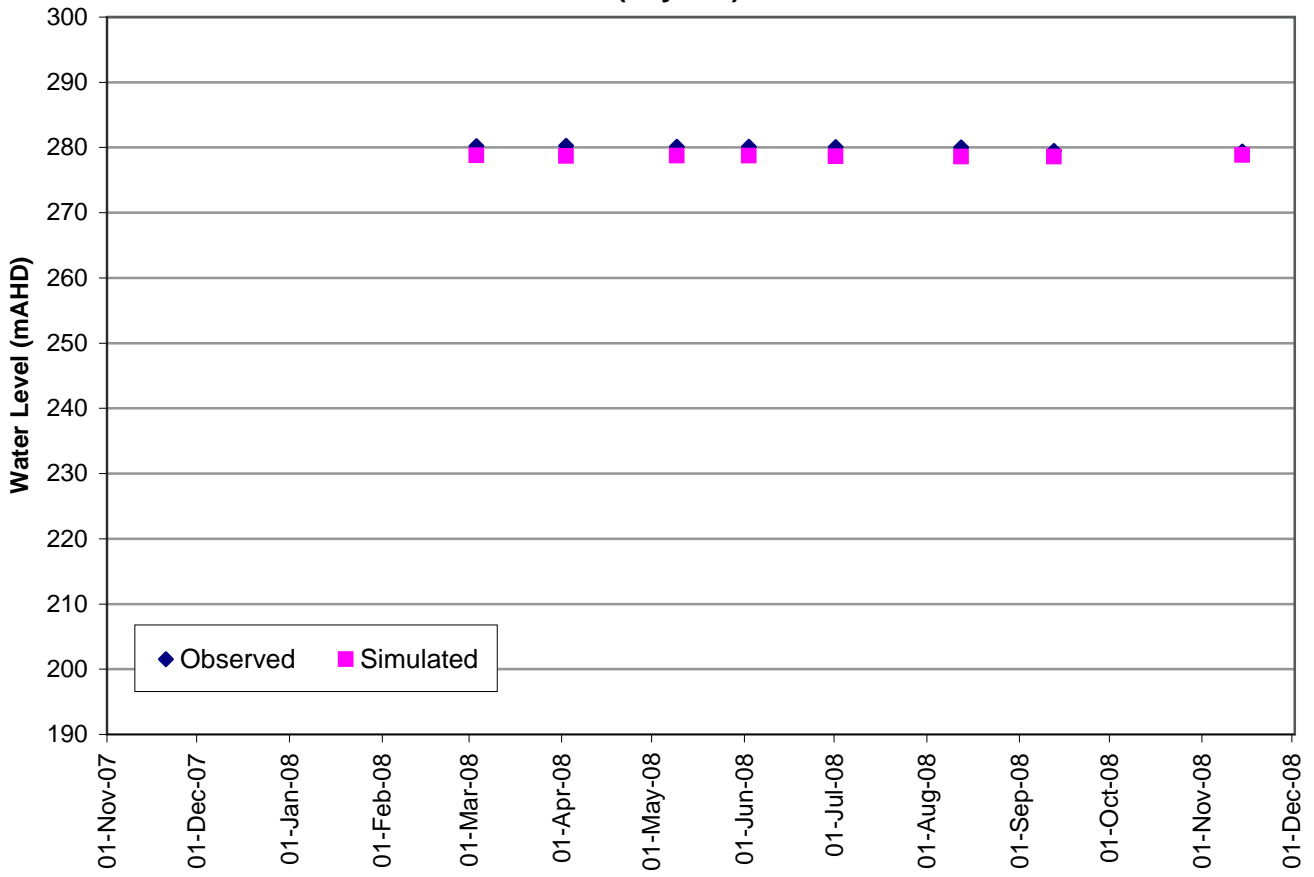
P6 (Layer 2)



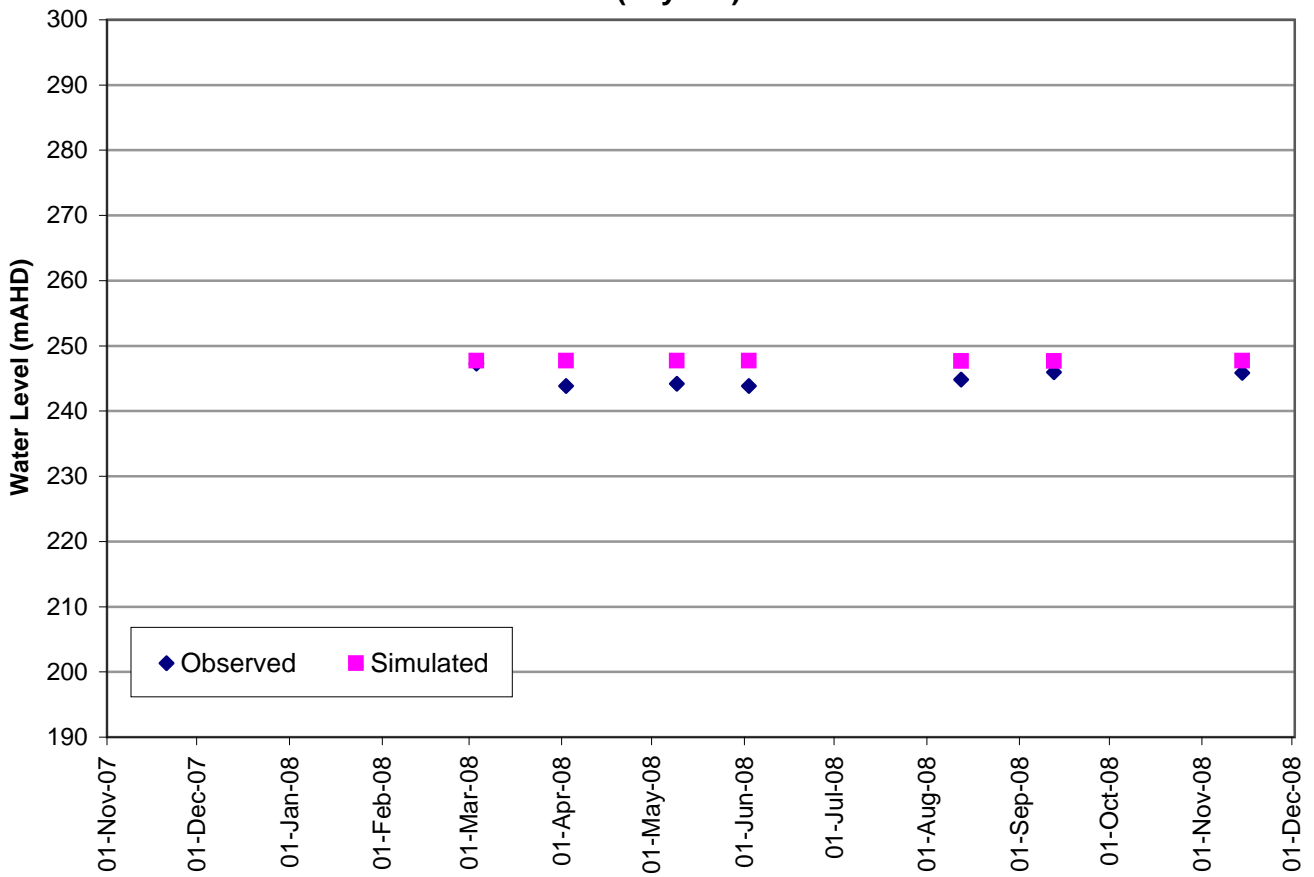
P9 (Layer 2)



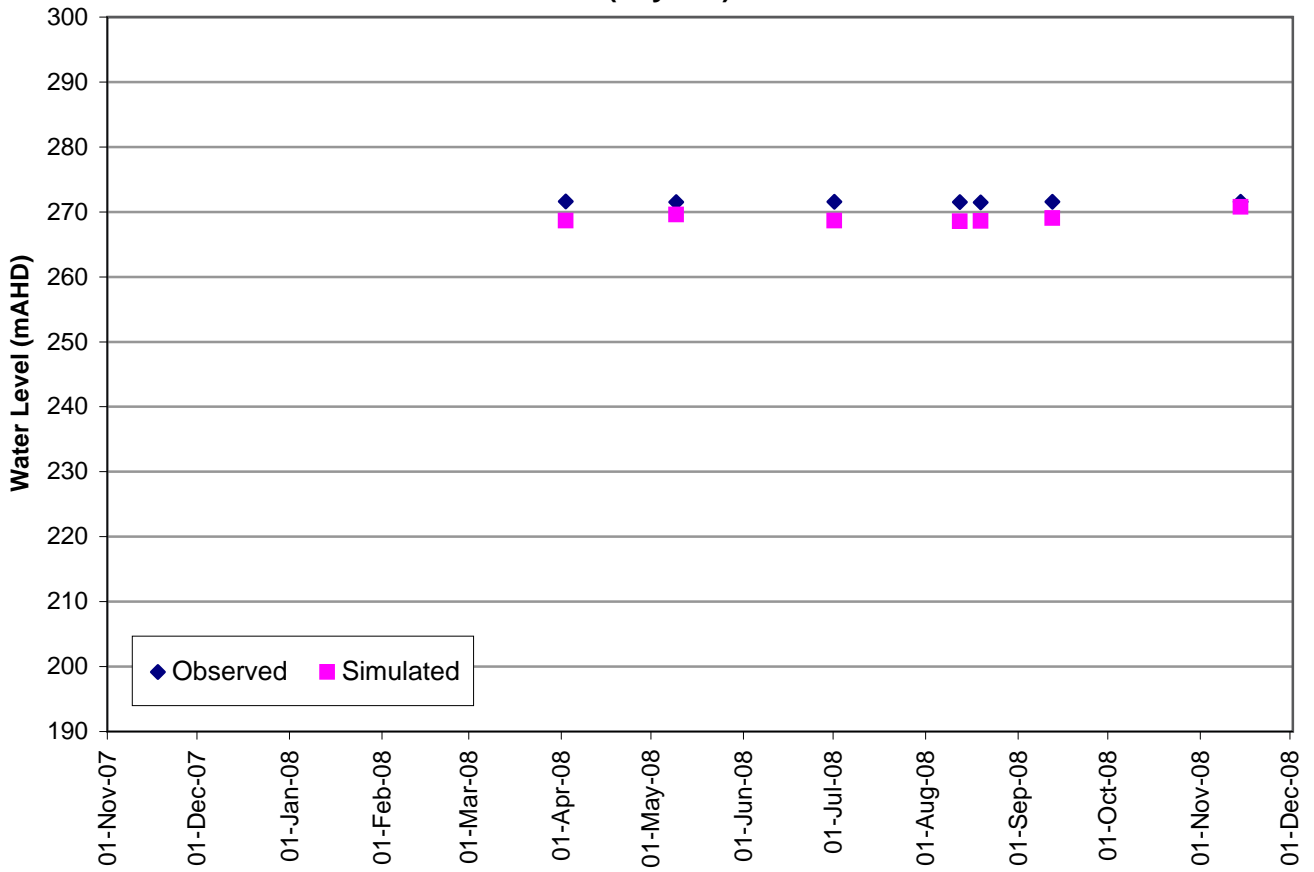
P11 (Layer 2)



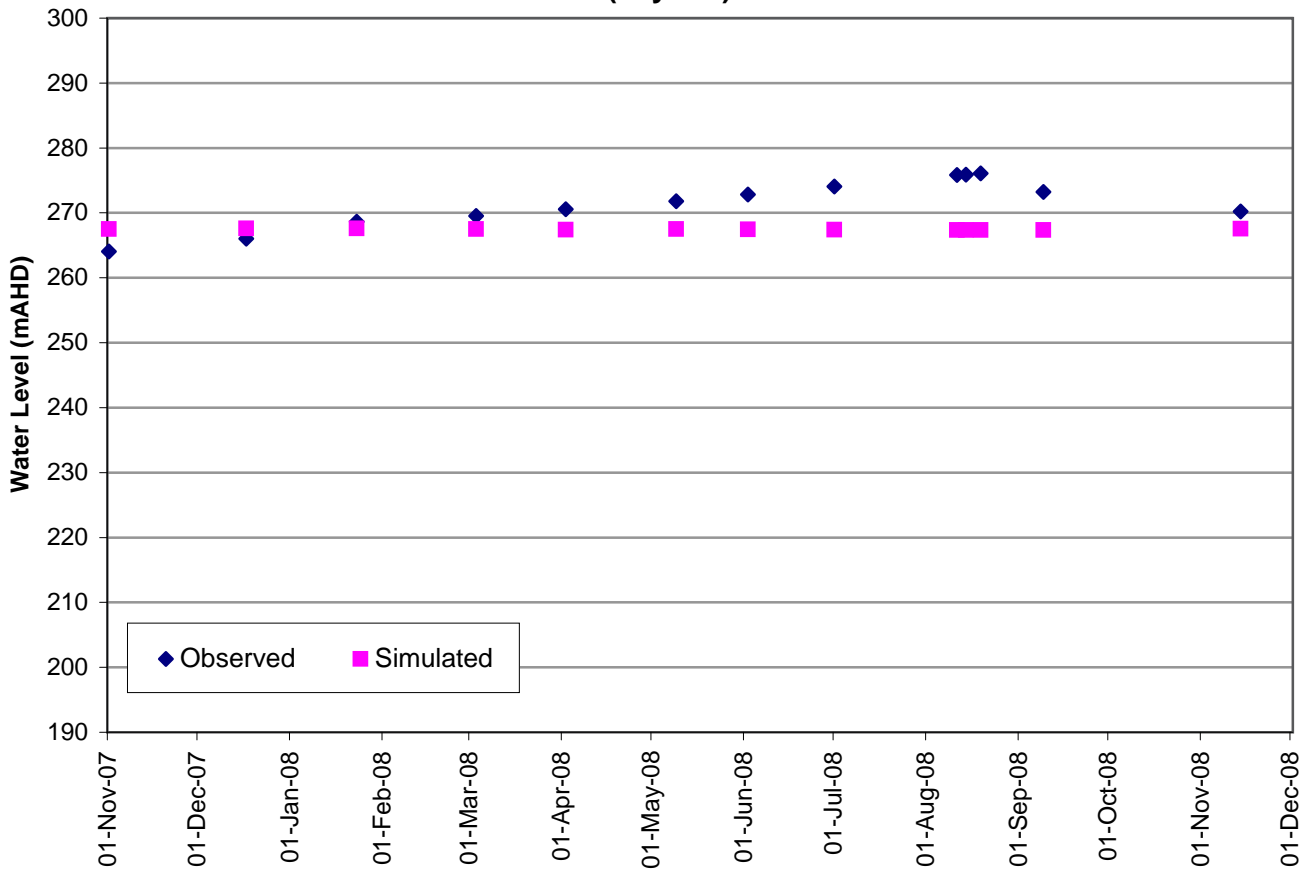
P17 (Layer 2)



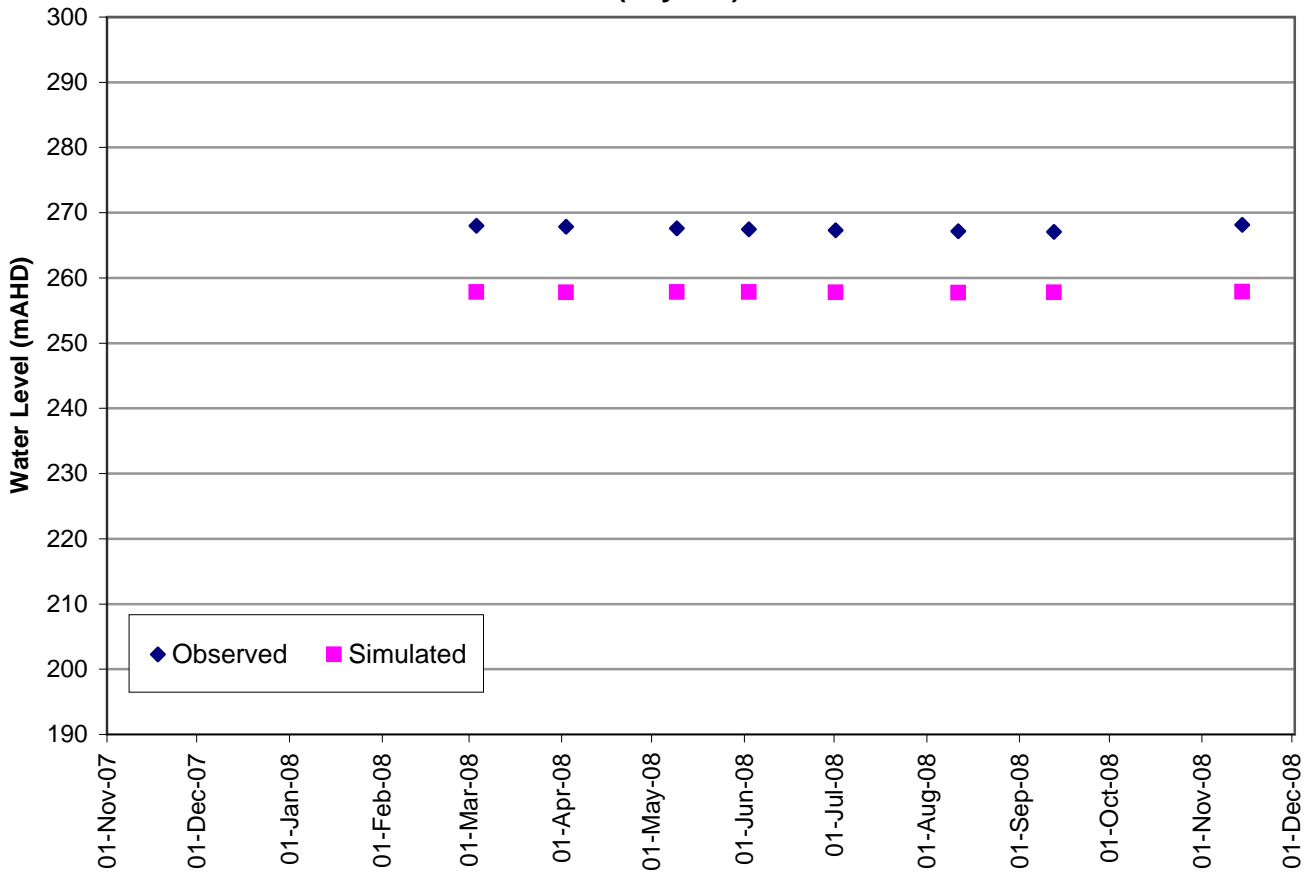
P8 (Layer 3)



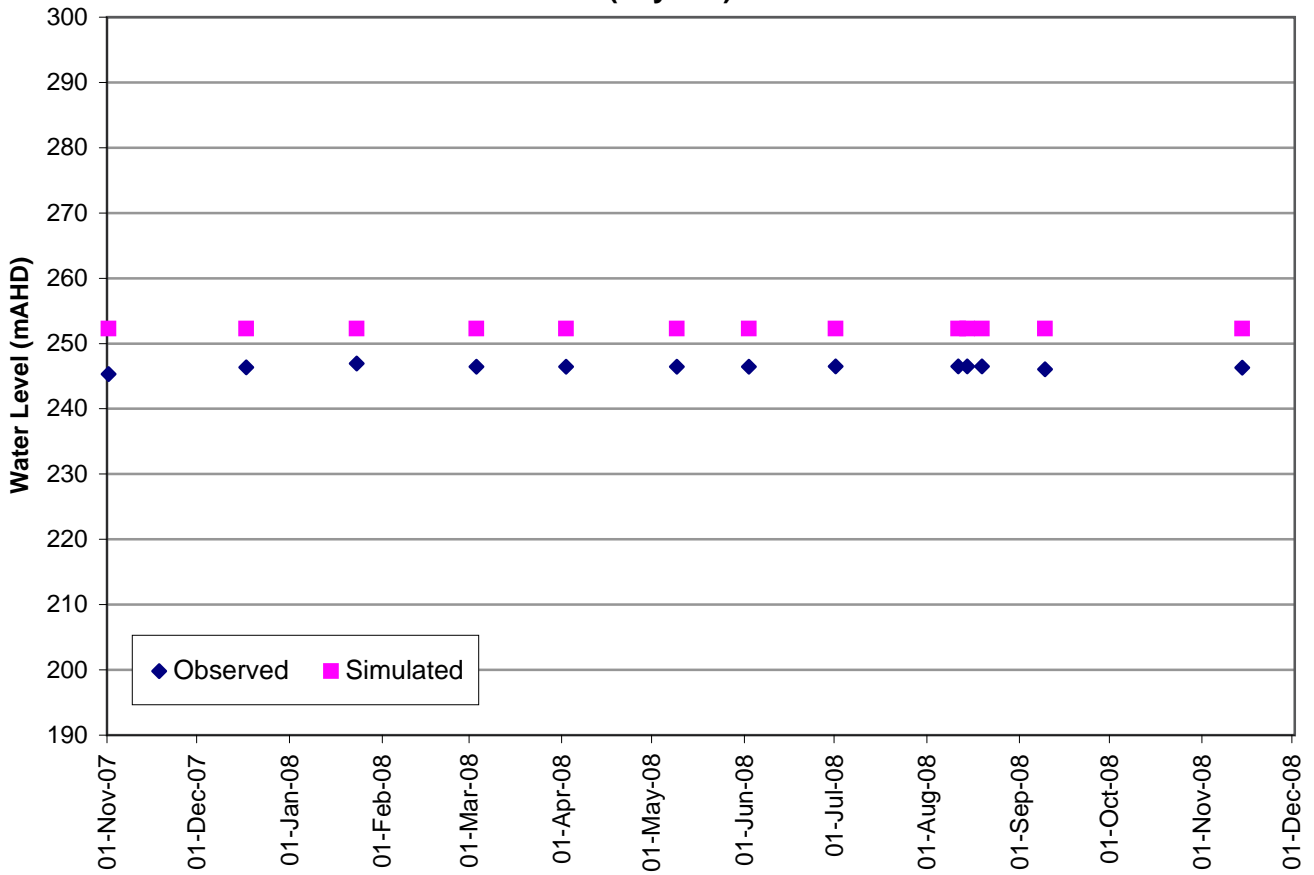
P1 (Layer 4)



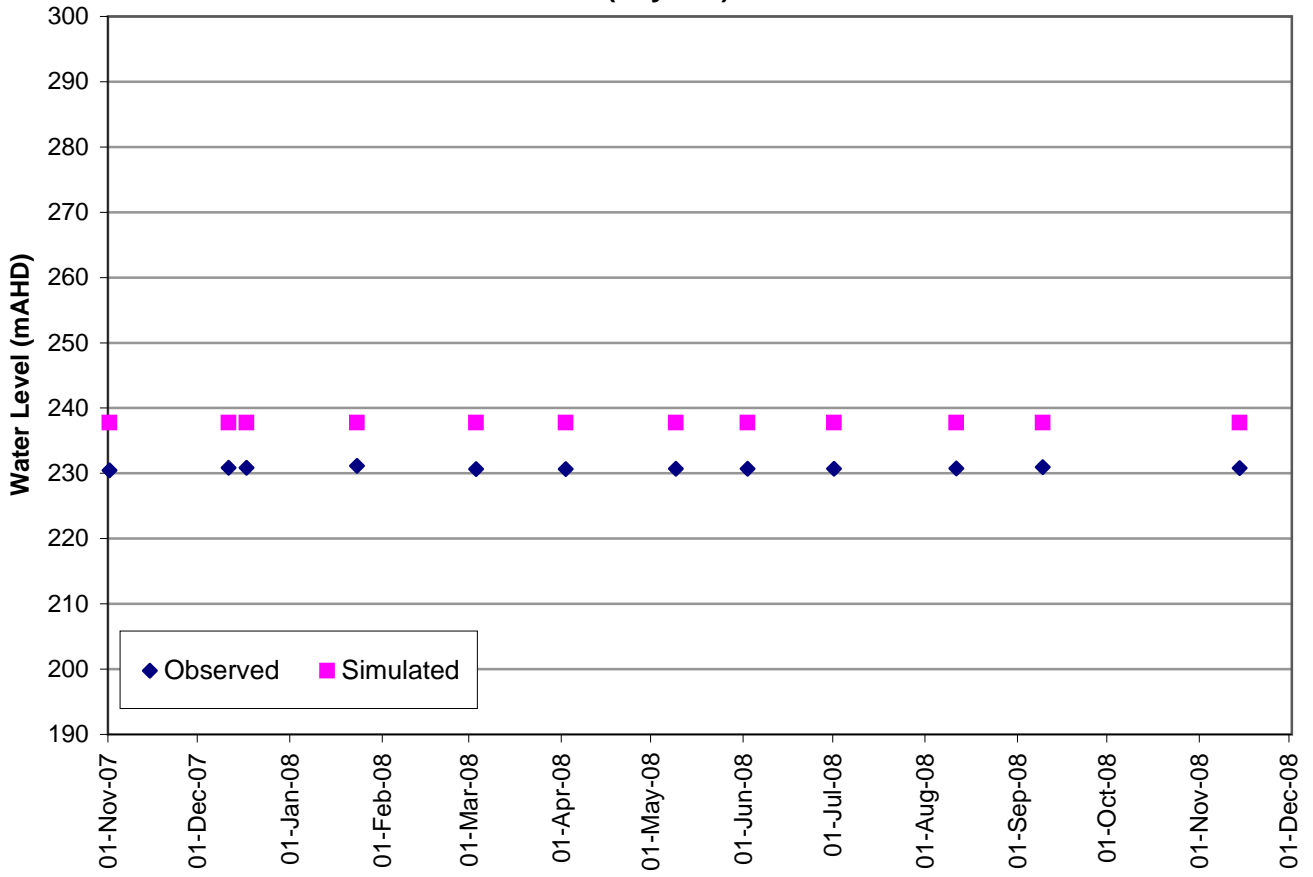
P13 (Layer 4)



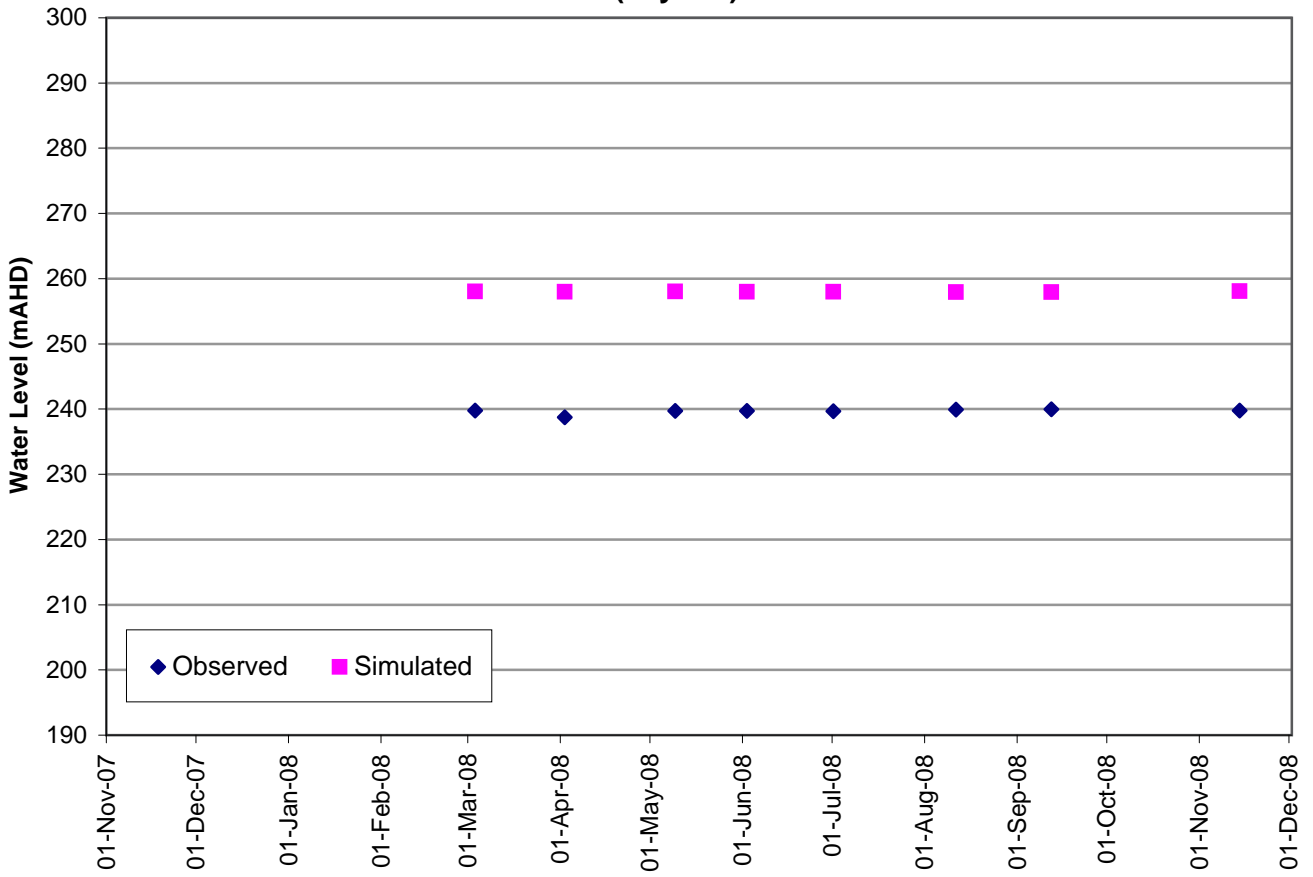
P2 (Layer 5)



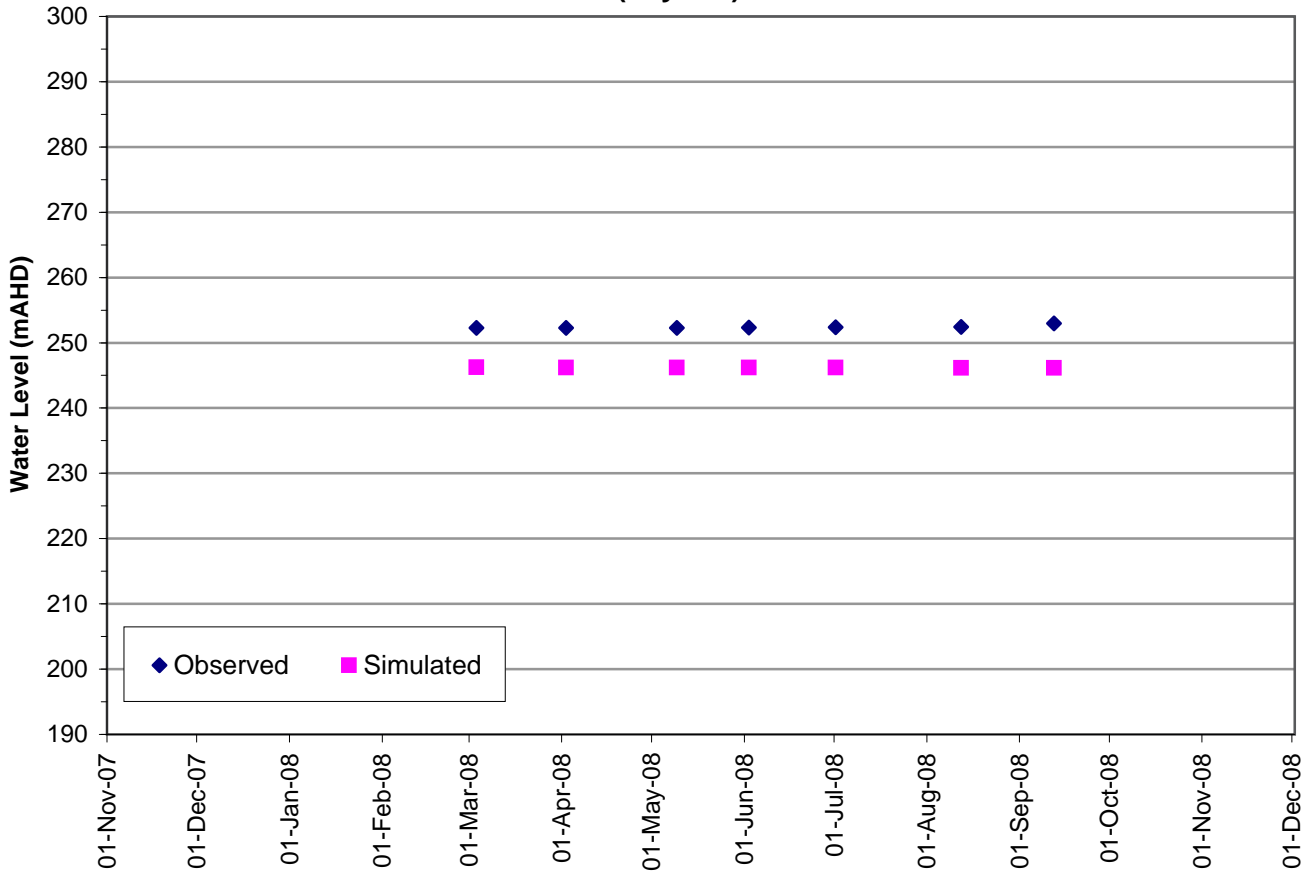
P4 (Layer 5)



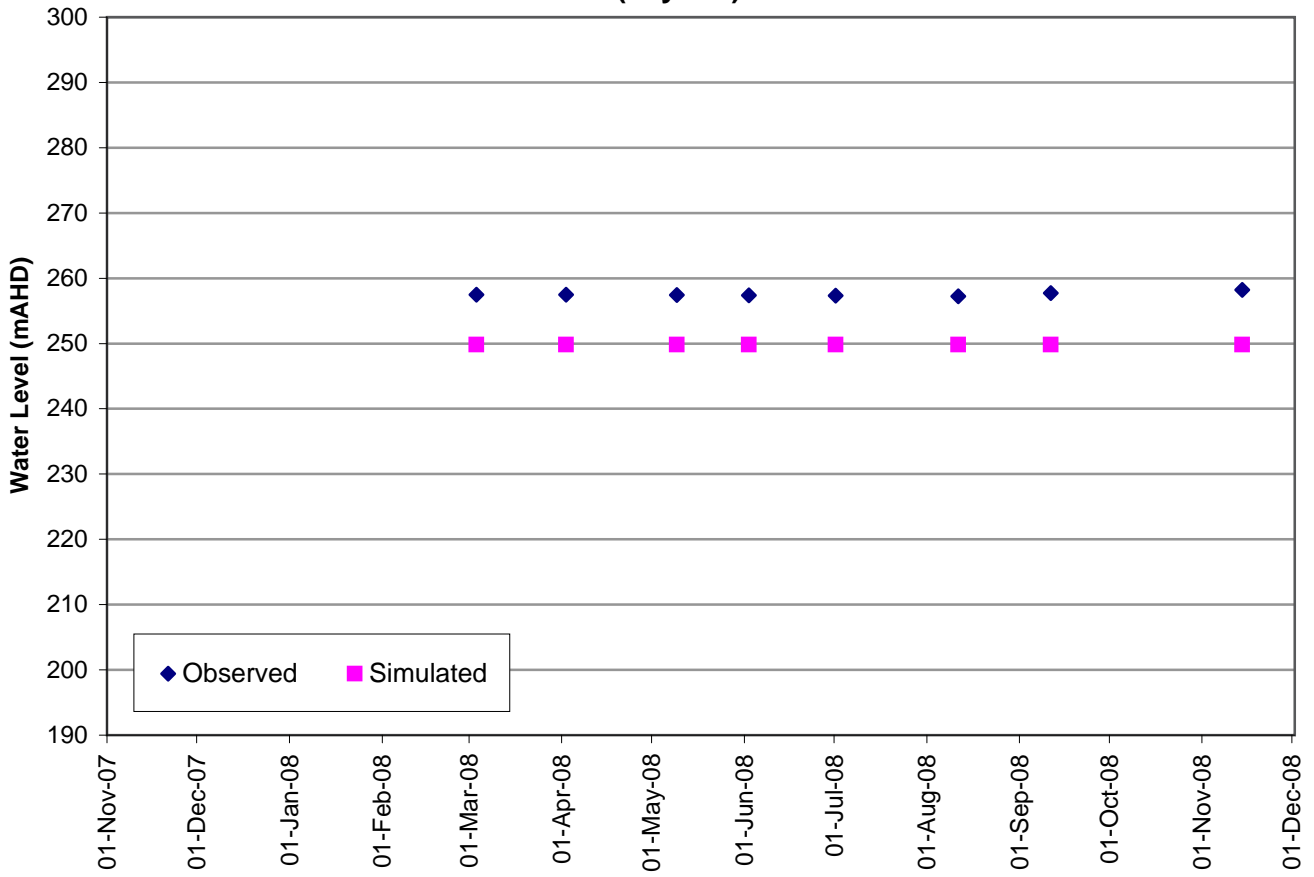
P12 (Layer 5)



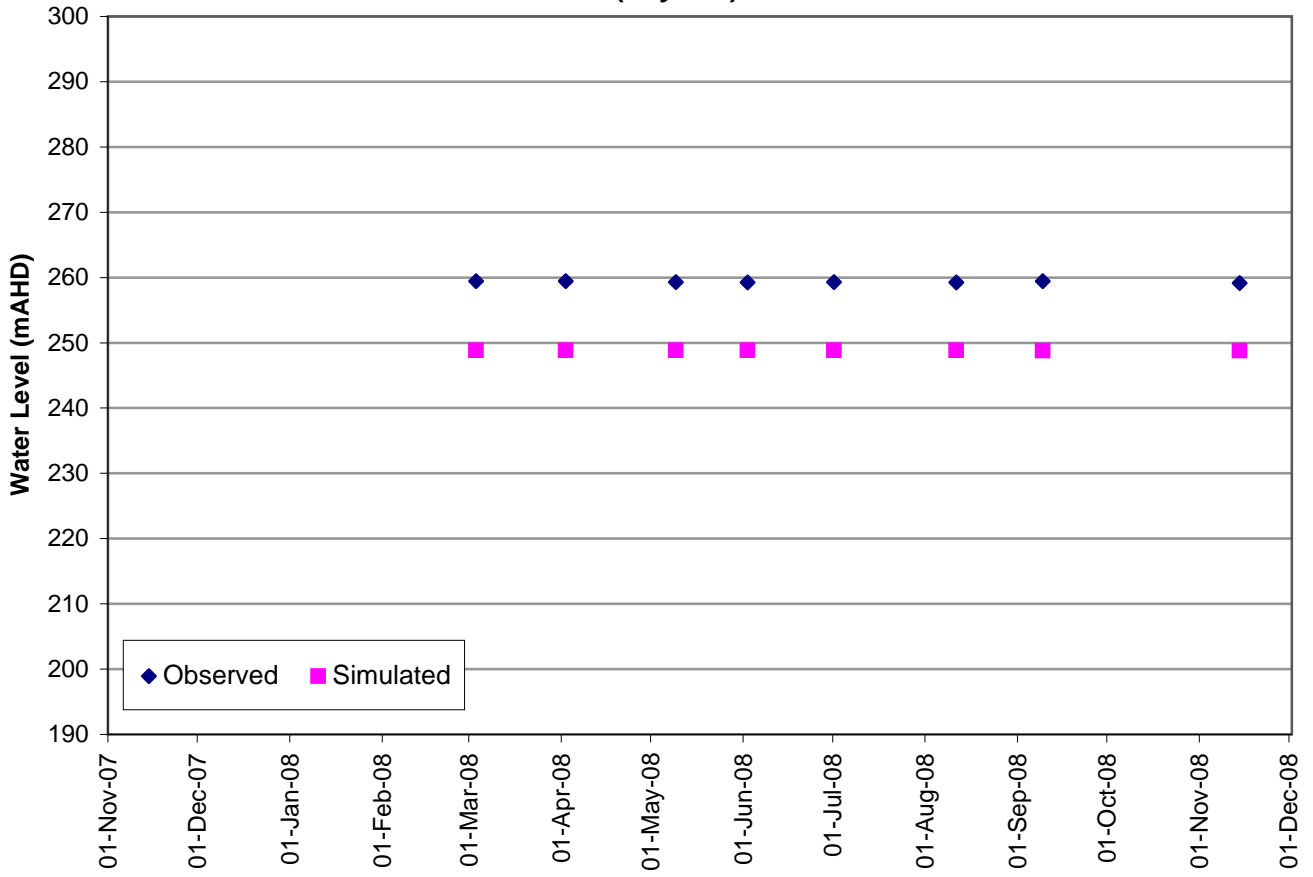
P16 (Layer 5)



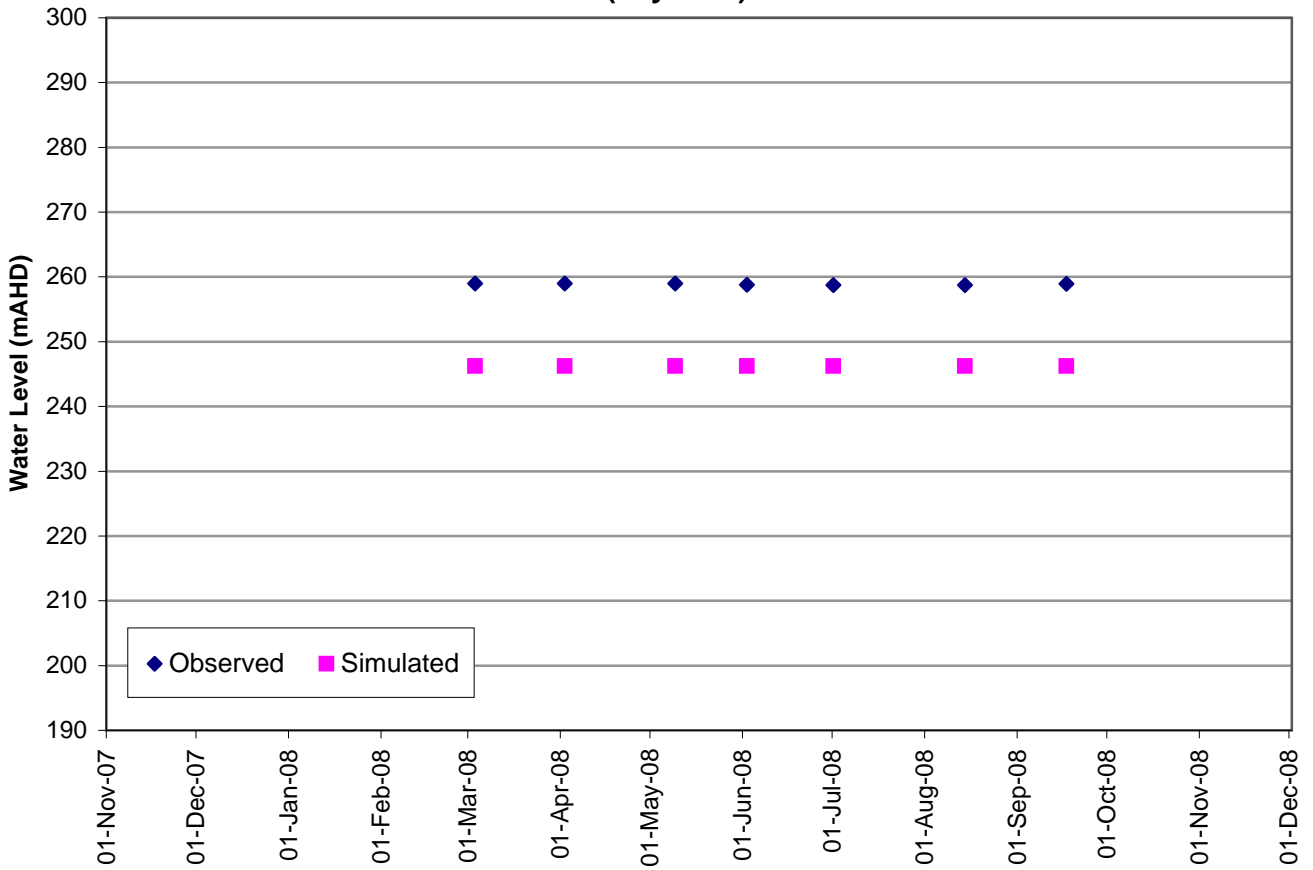
P18 (Layer 8)



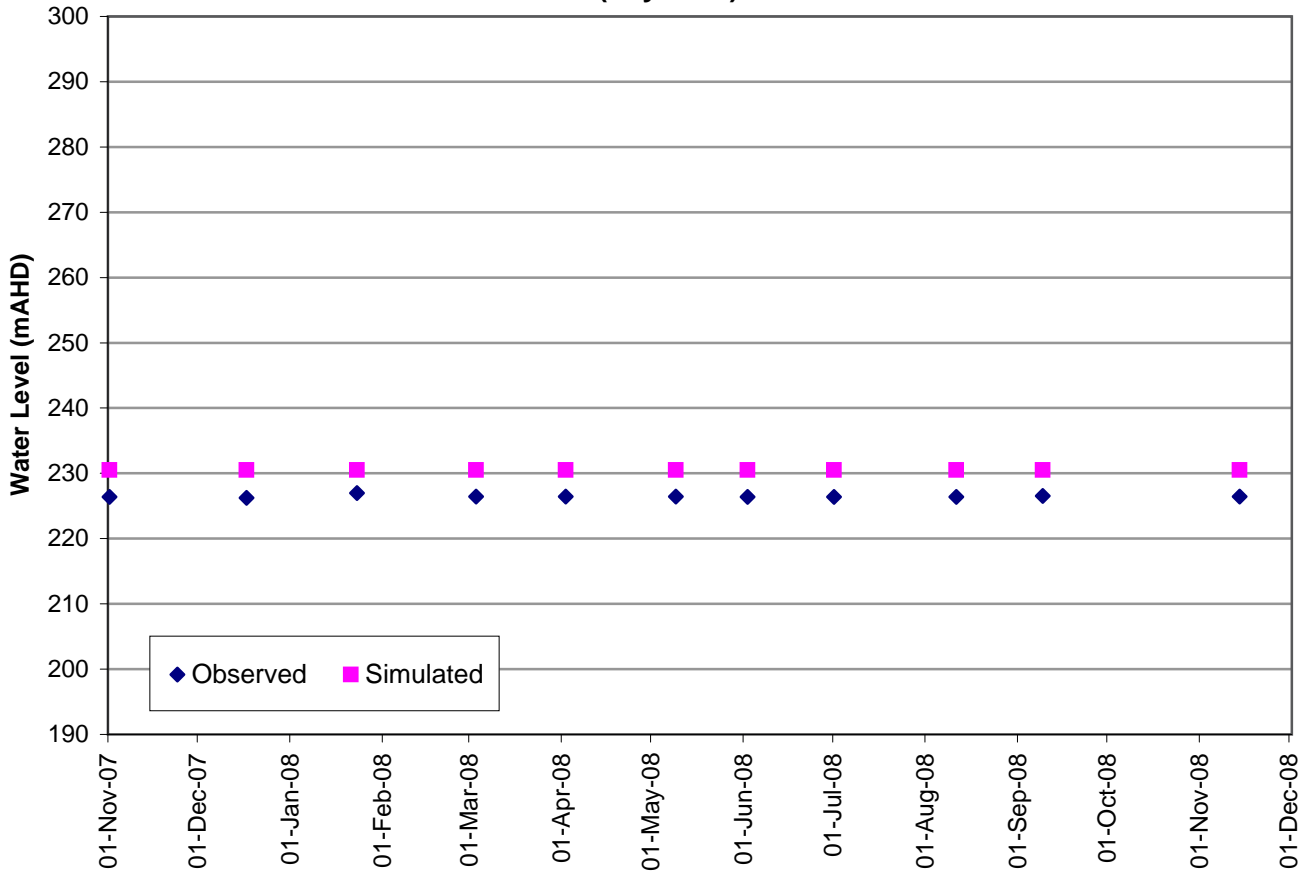
P20 (Layer 9)



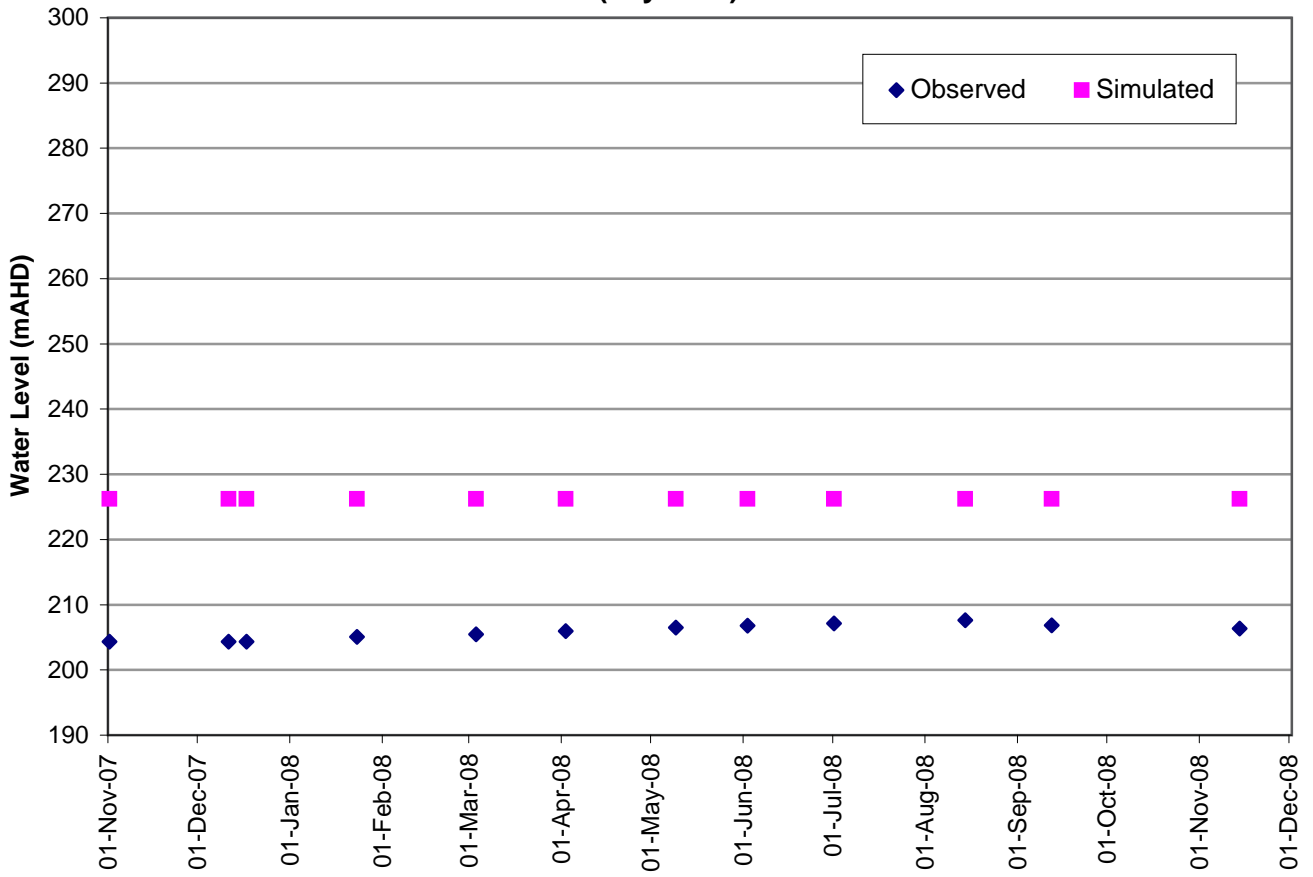
P19 (Layer 10)



P3 (Layer 11)

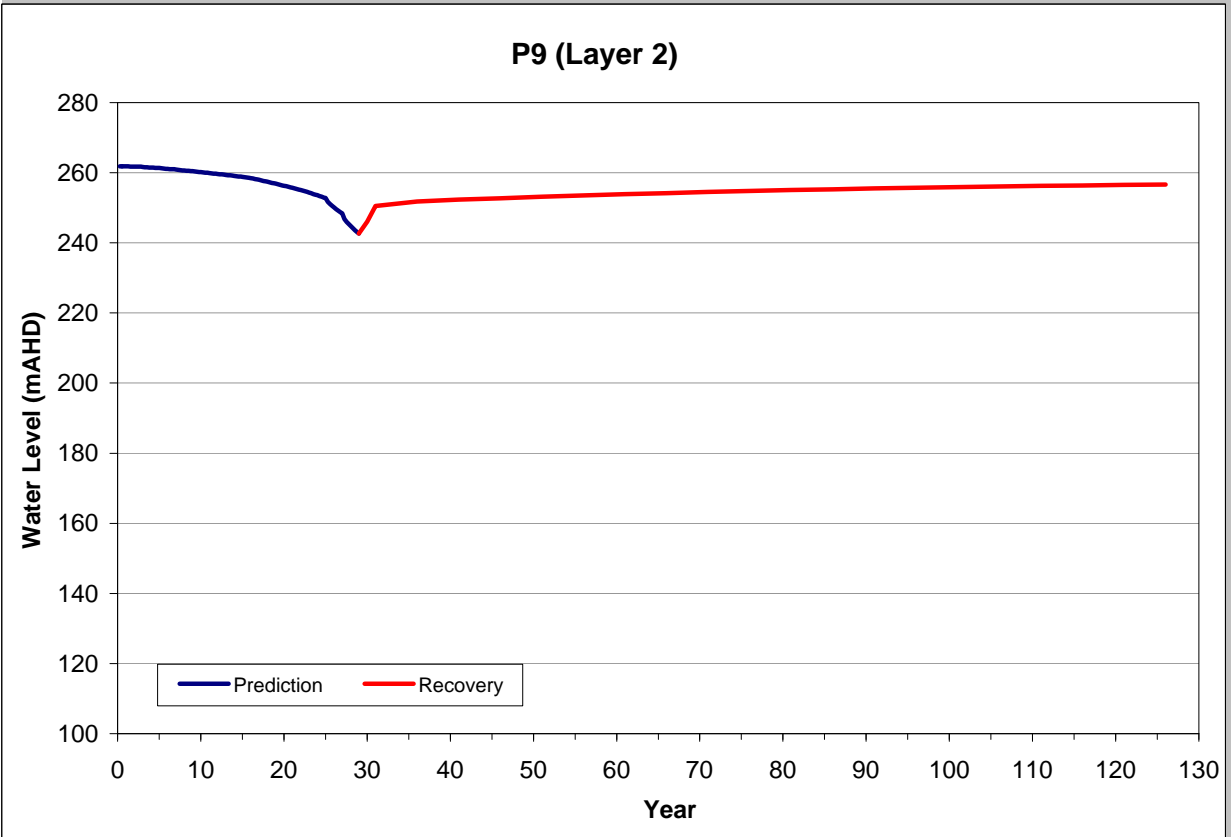
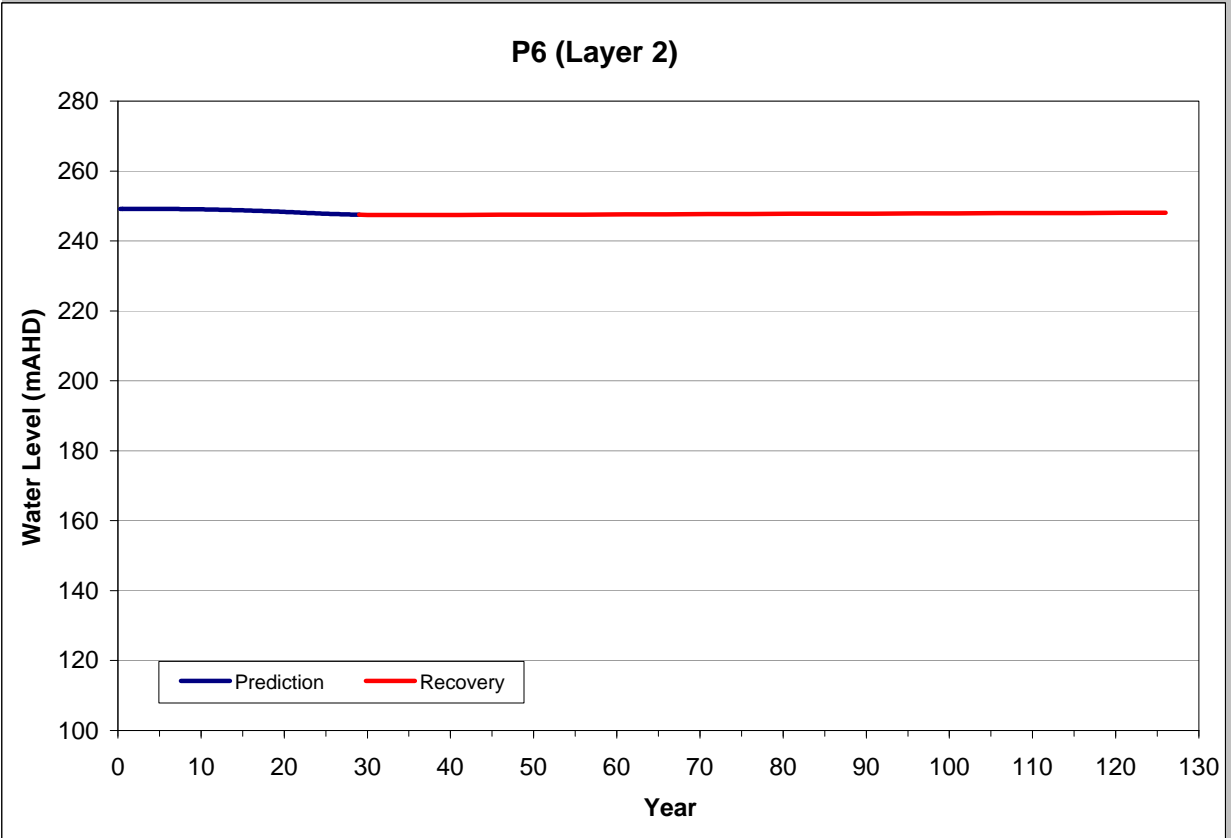


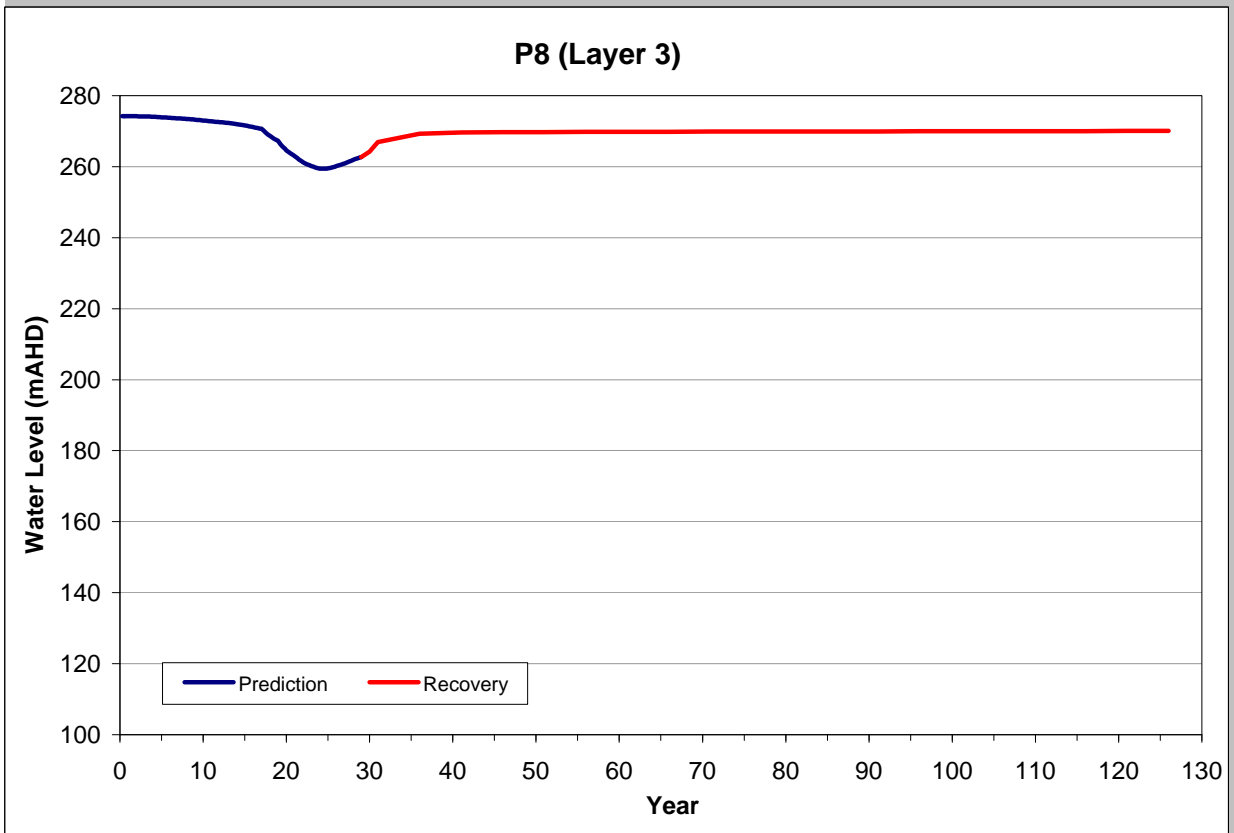
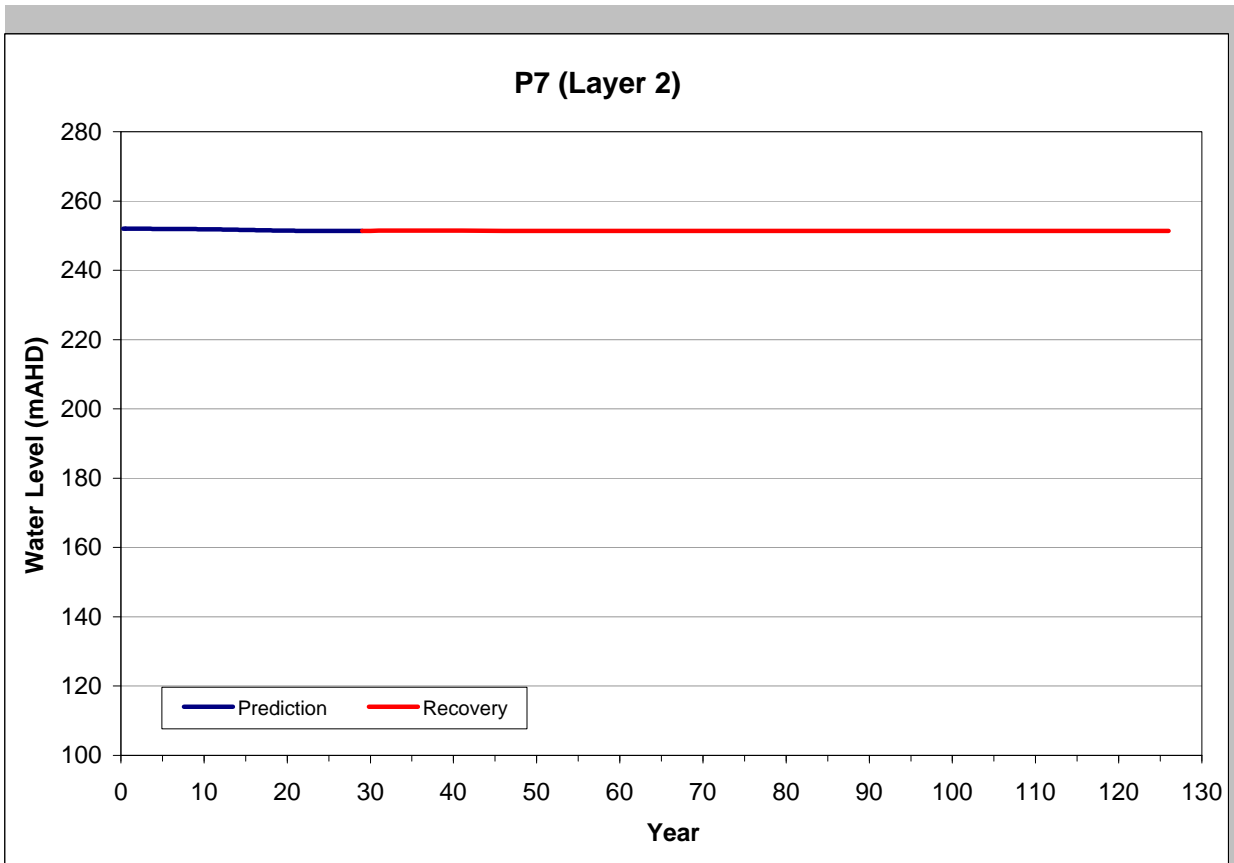
P5 (Layer 11)

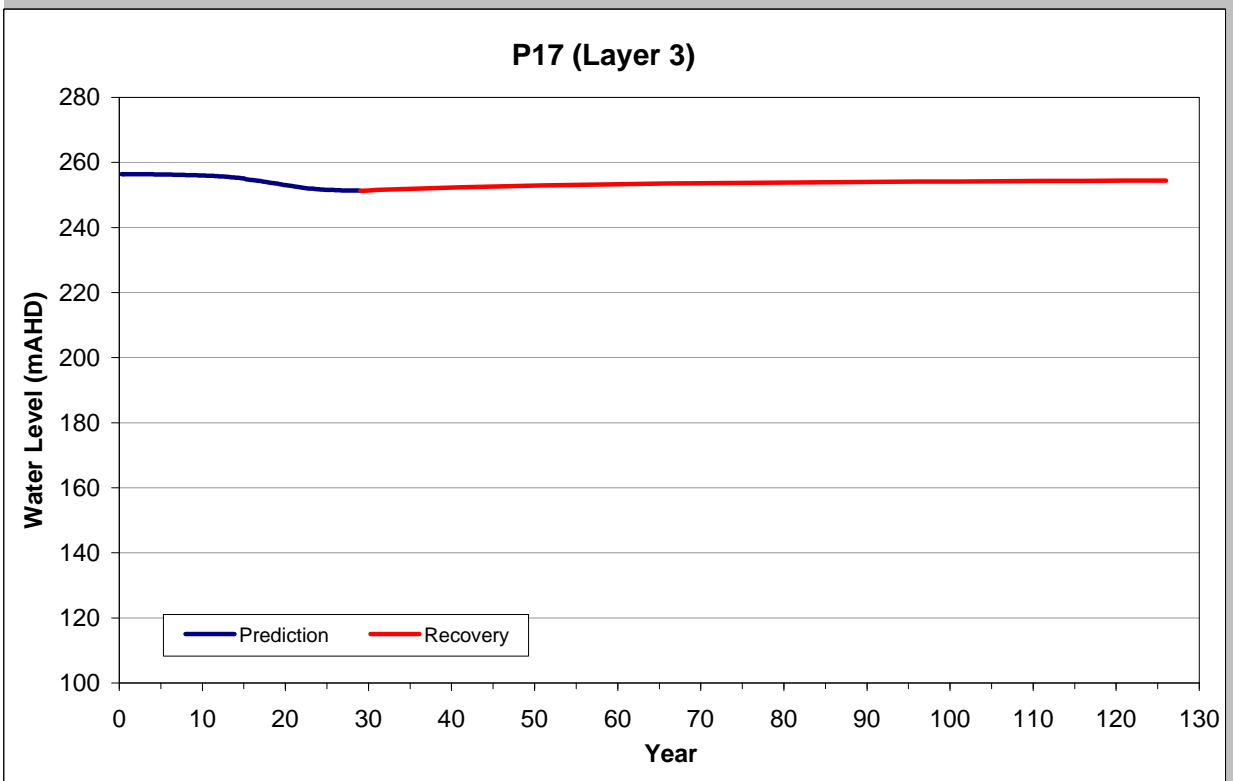
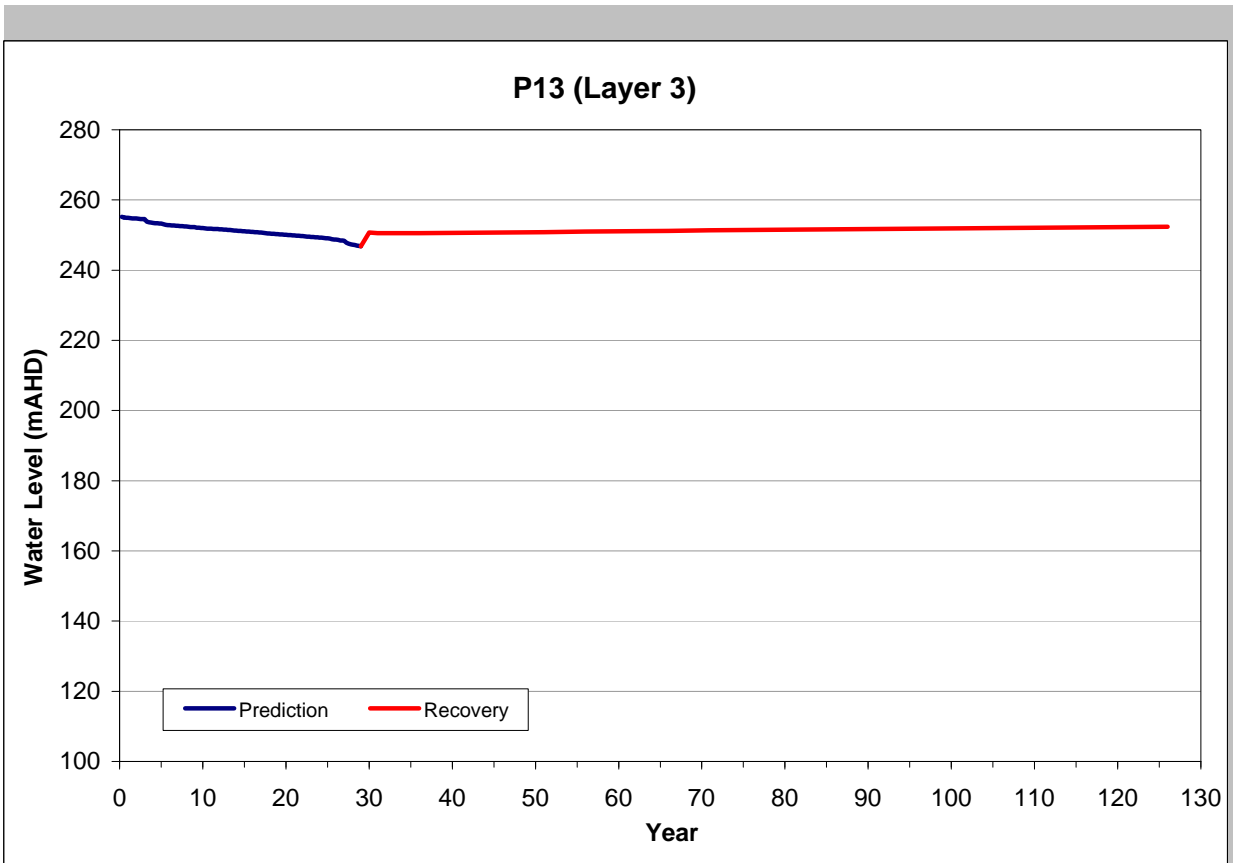


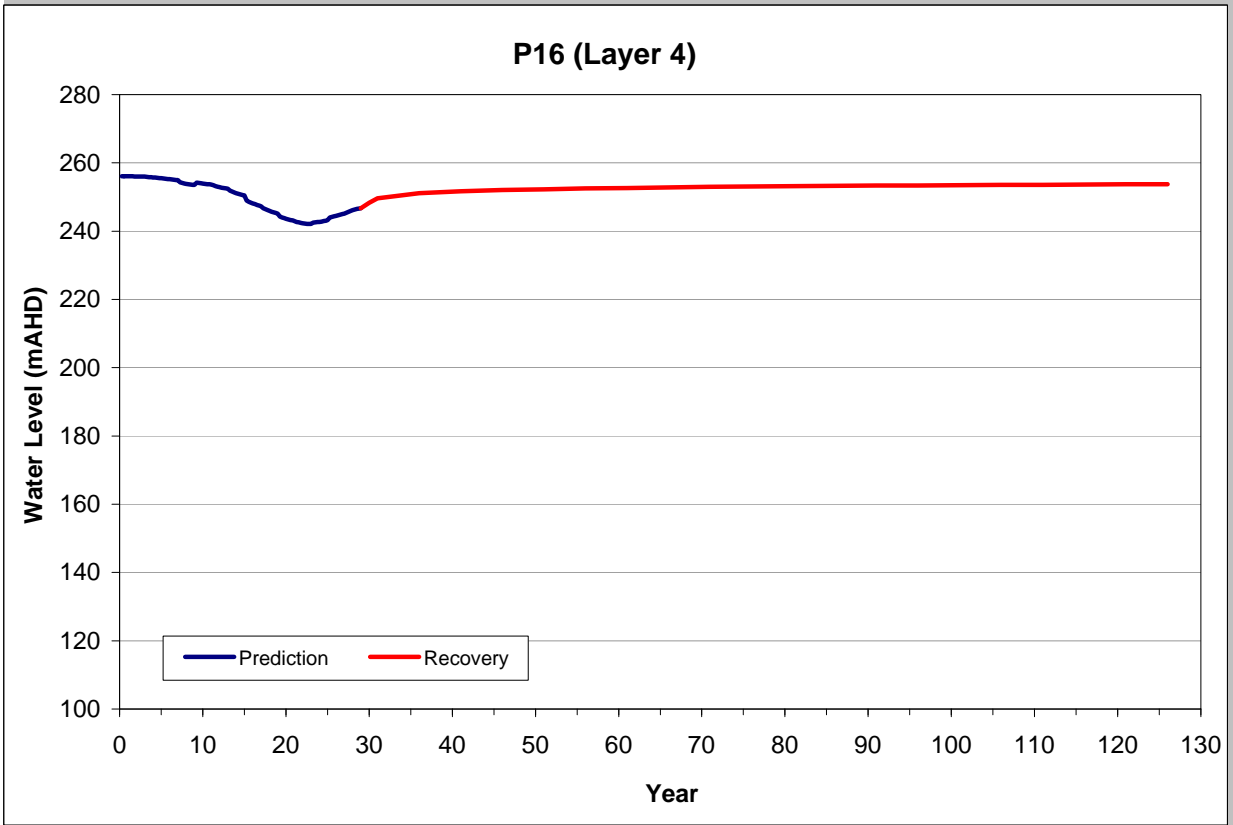
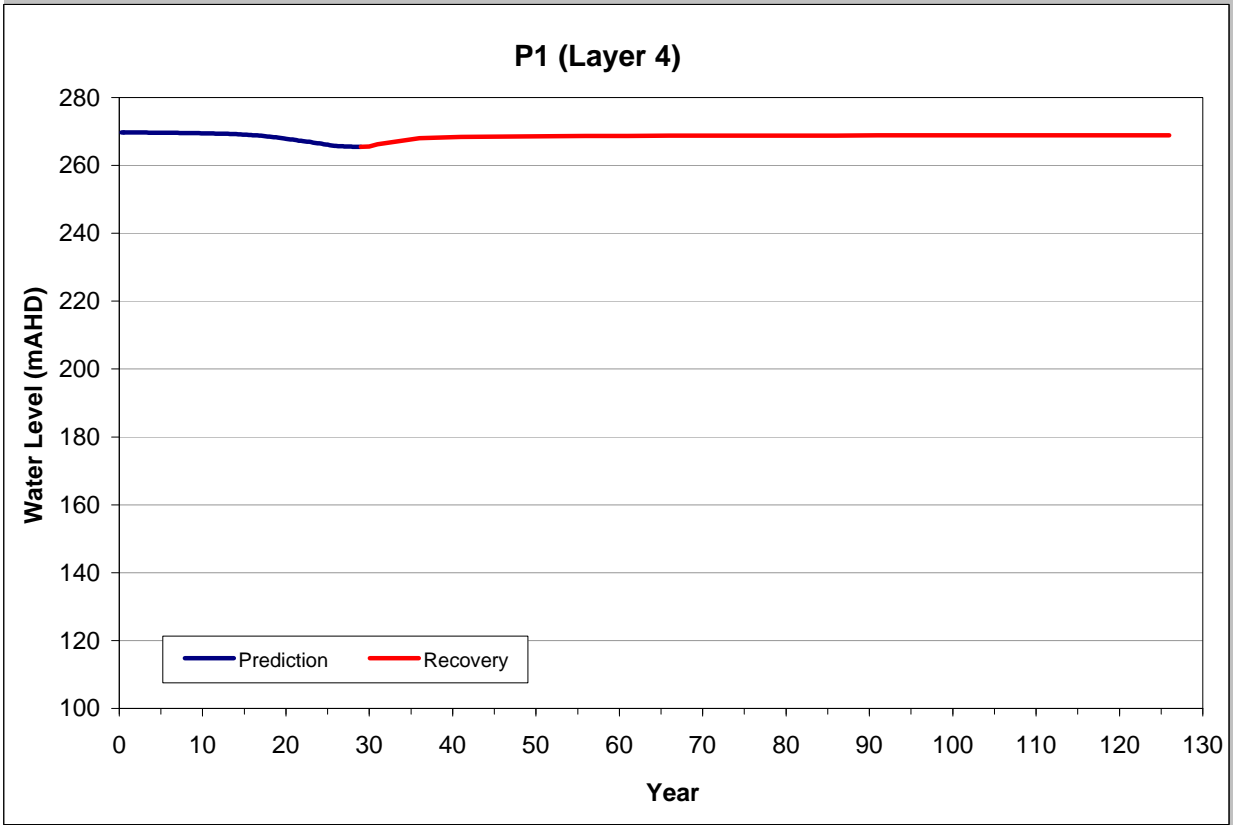
APPENDIX H

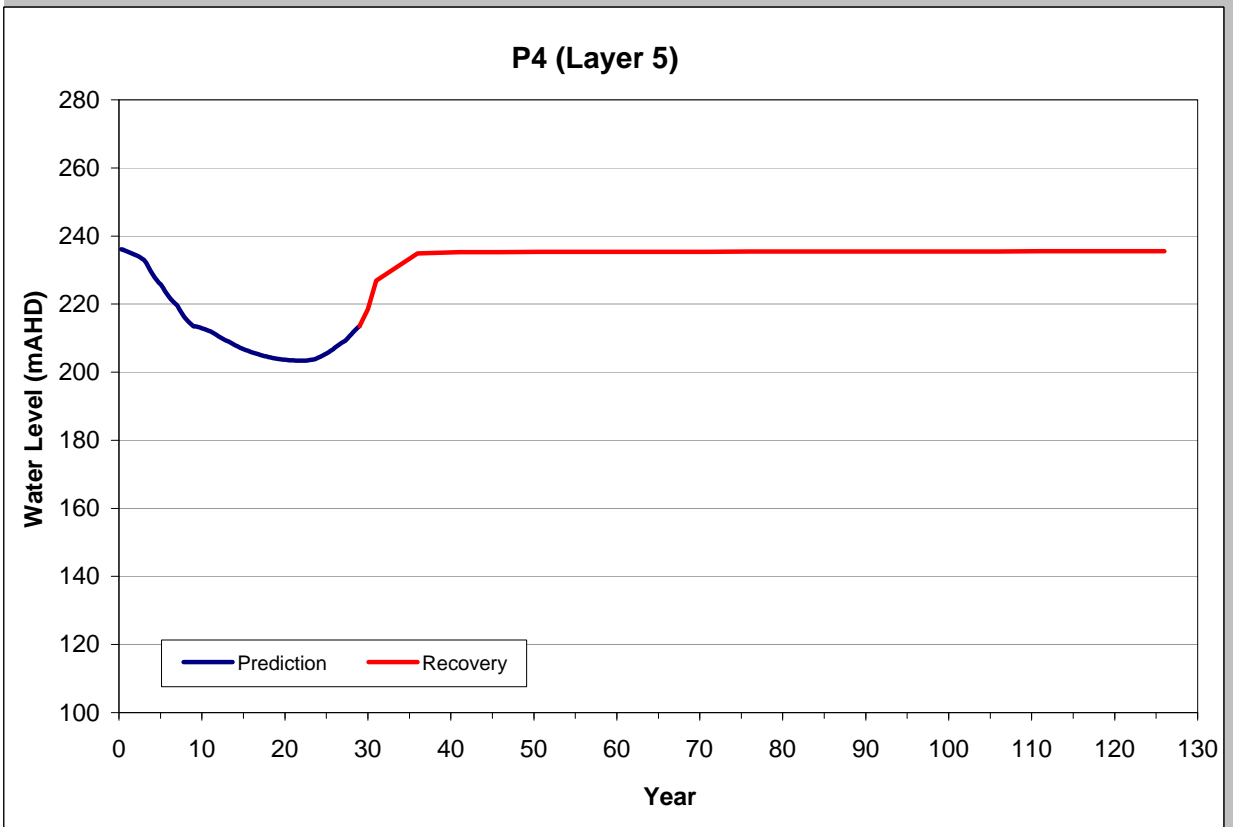
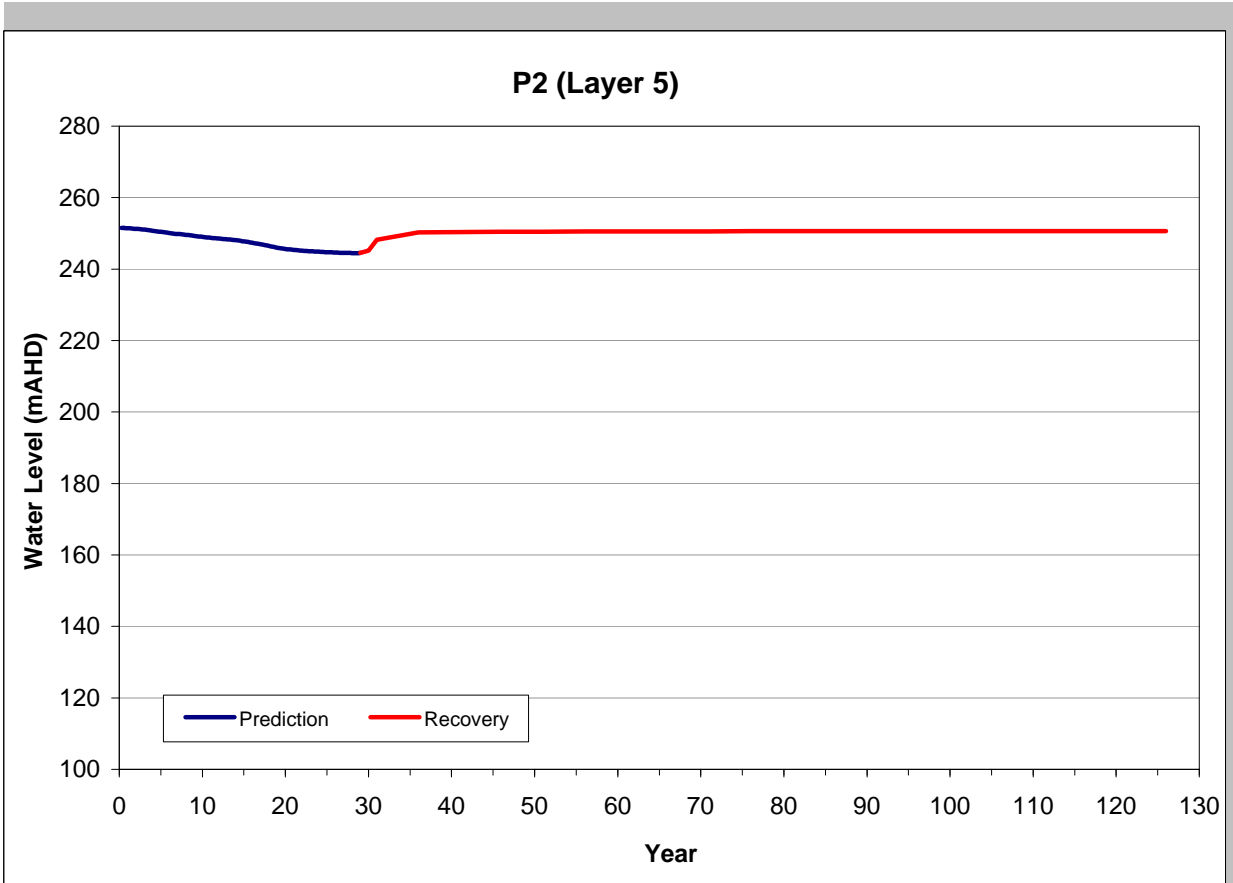
NARRABRI PREDICTED RECOVERY HYDRAGRAPH

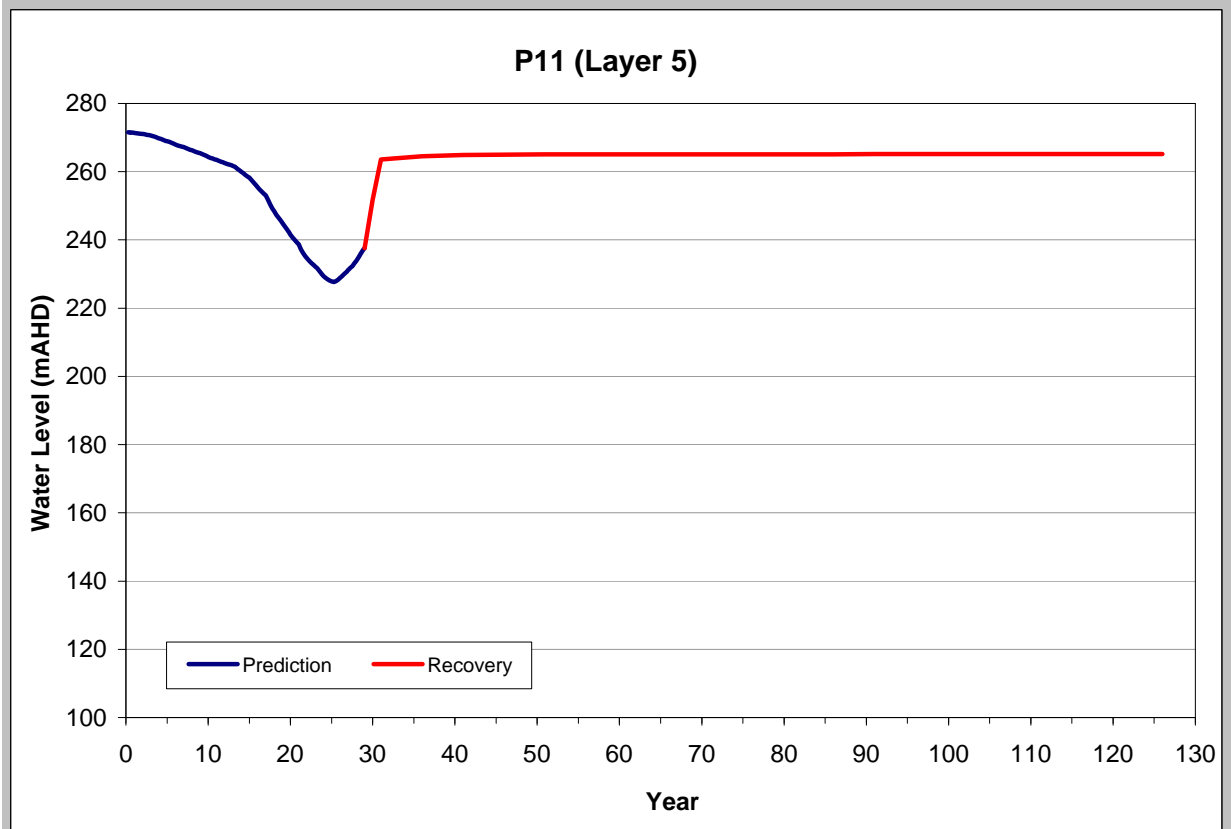
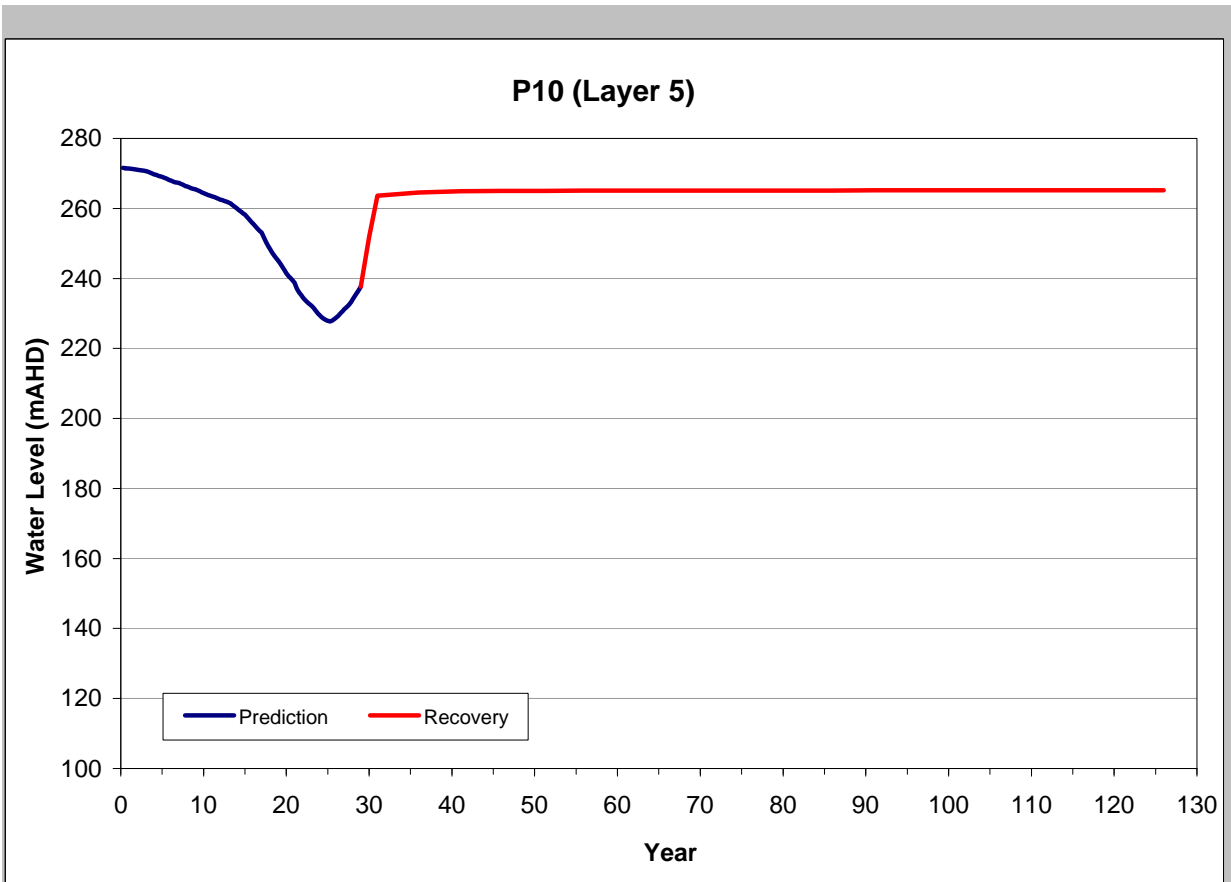


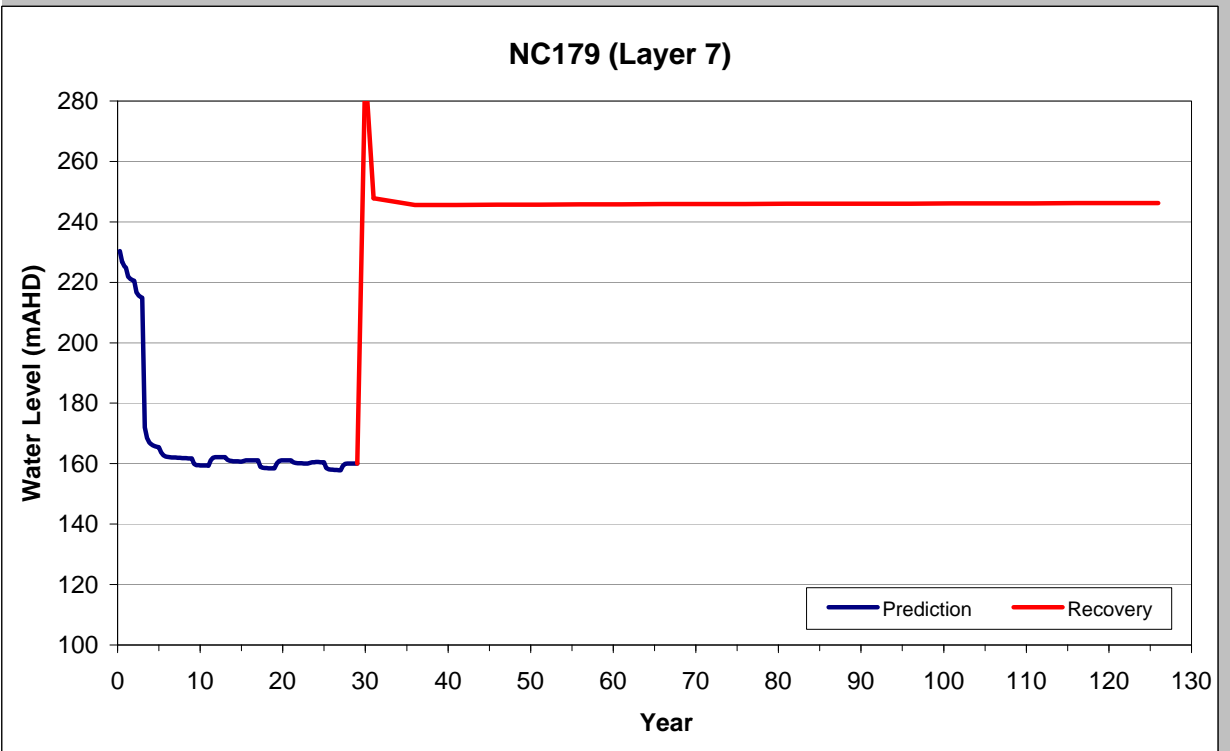
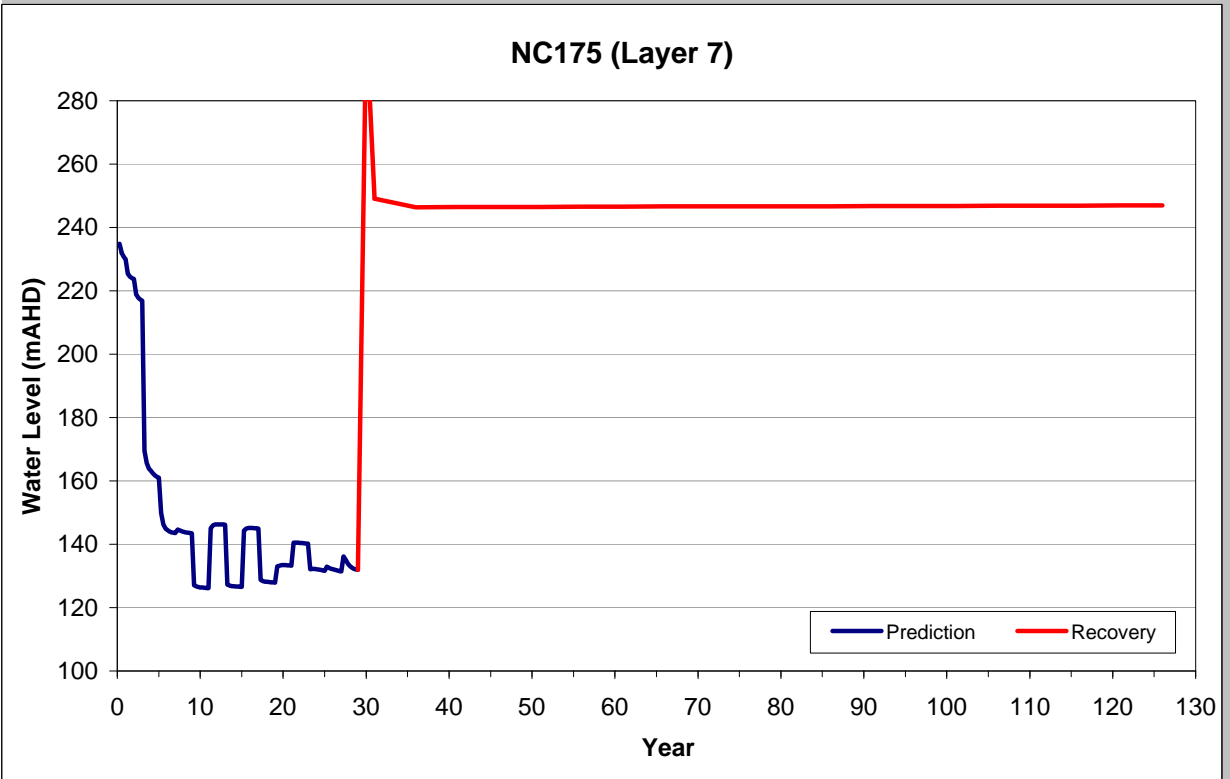


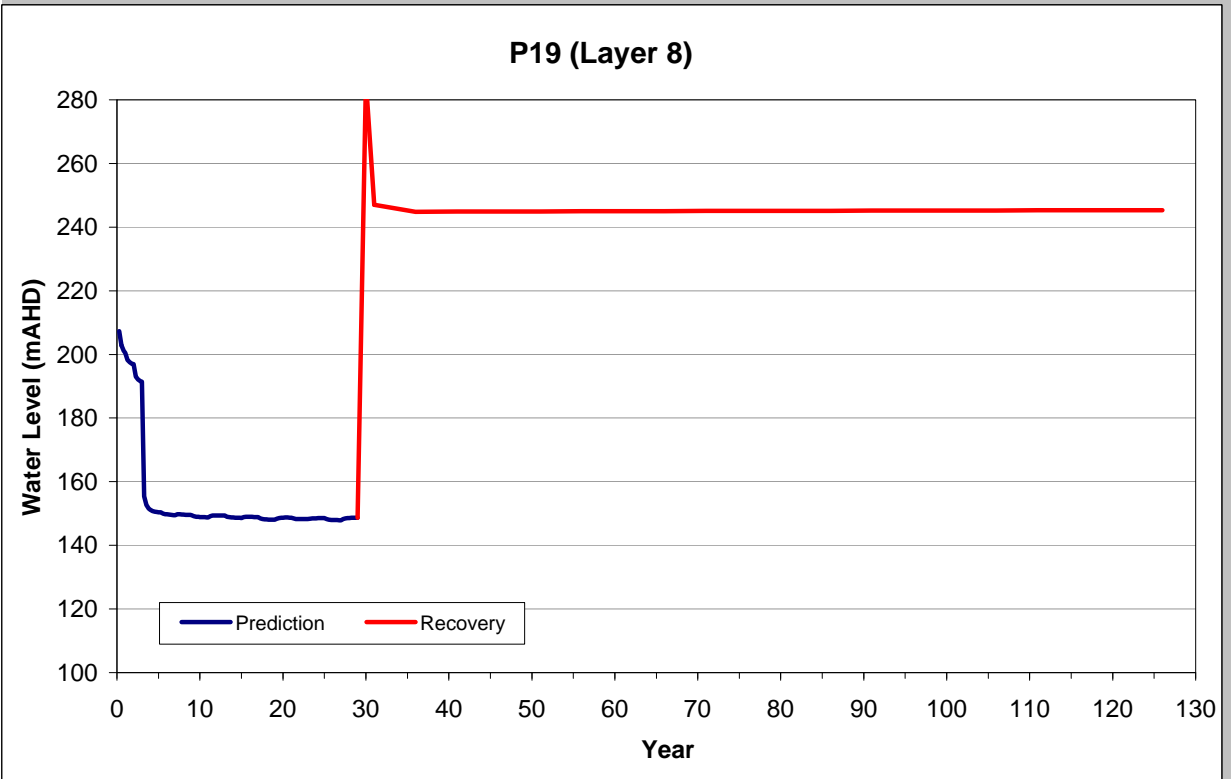
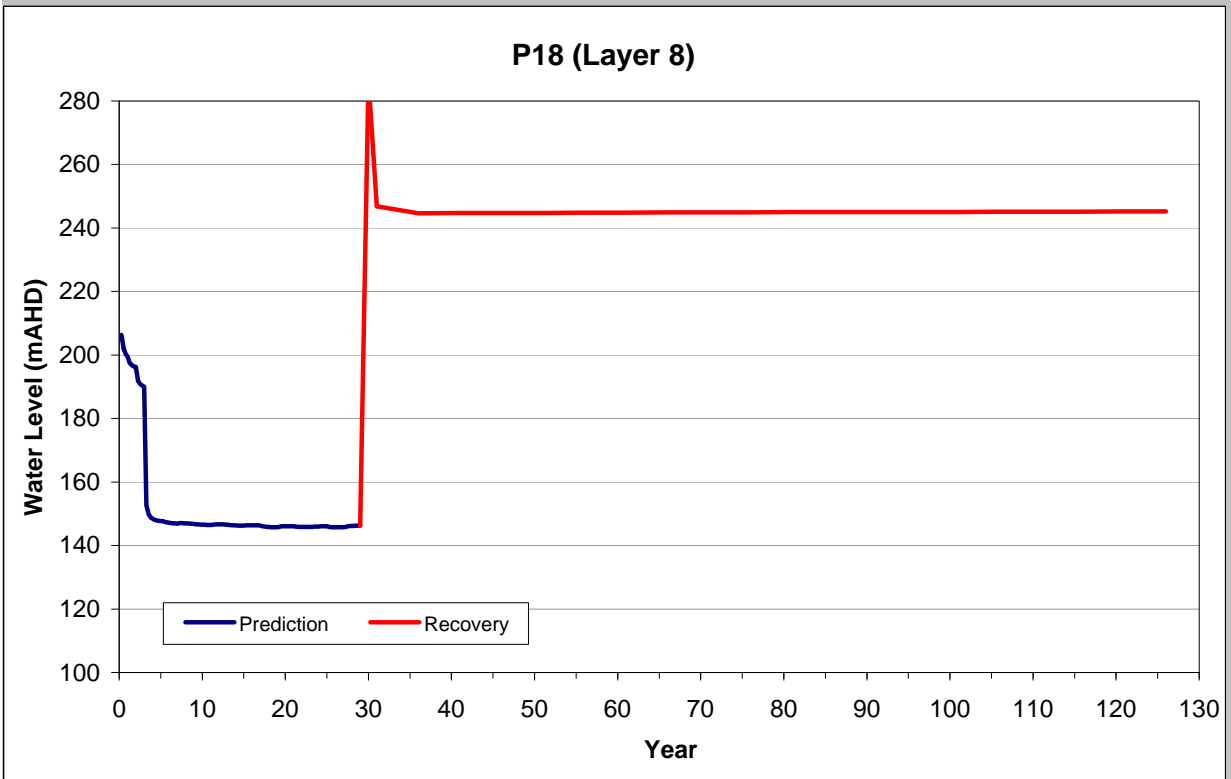


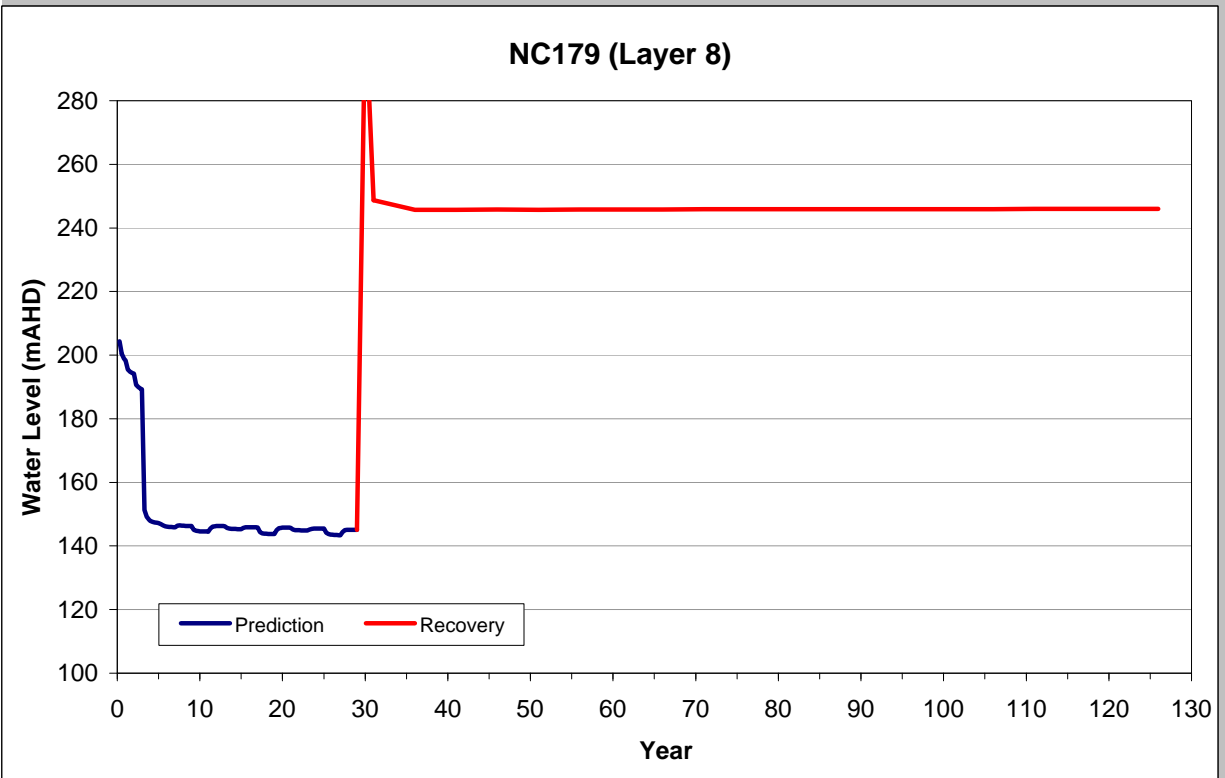
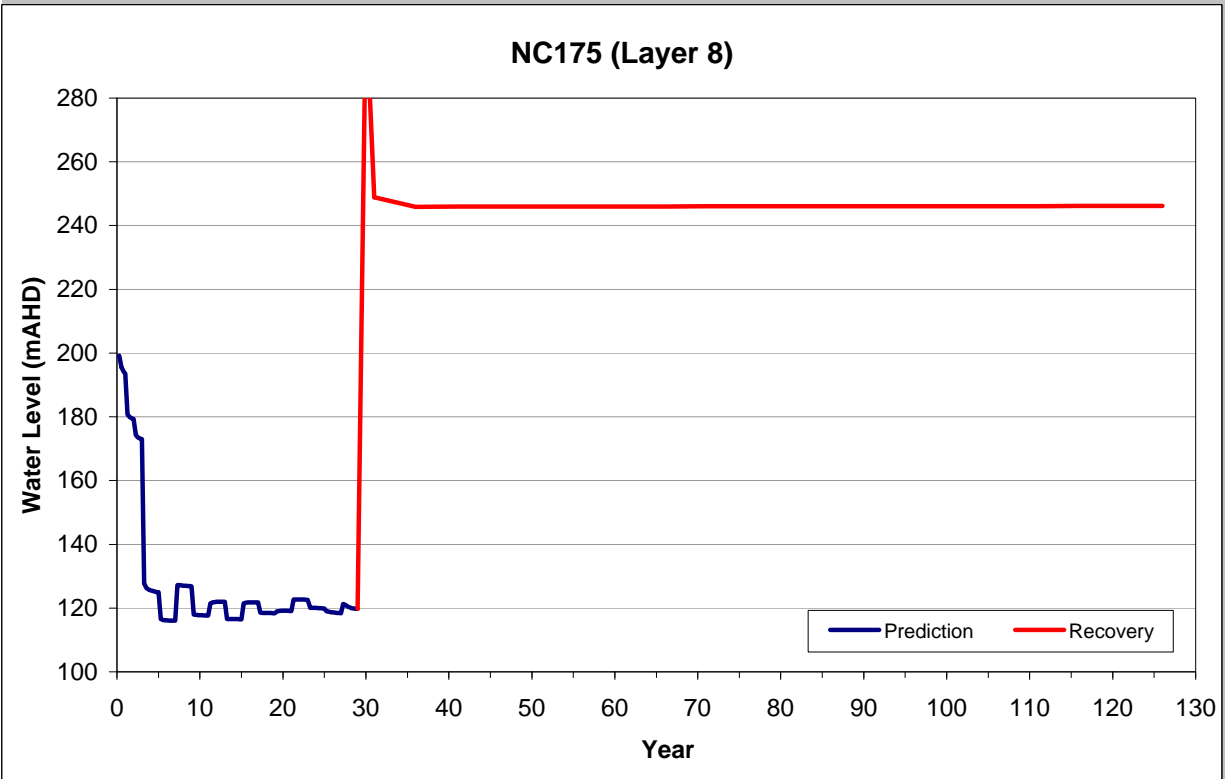


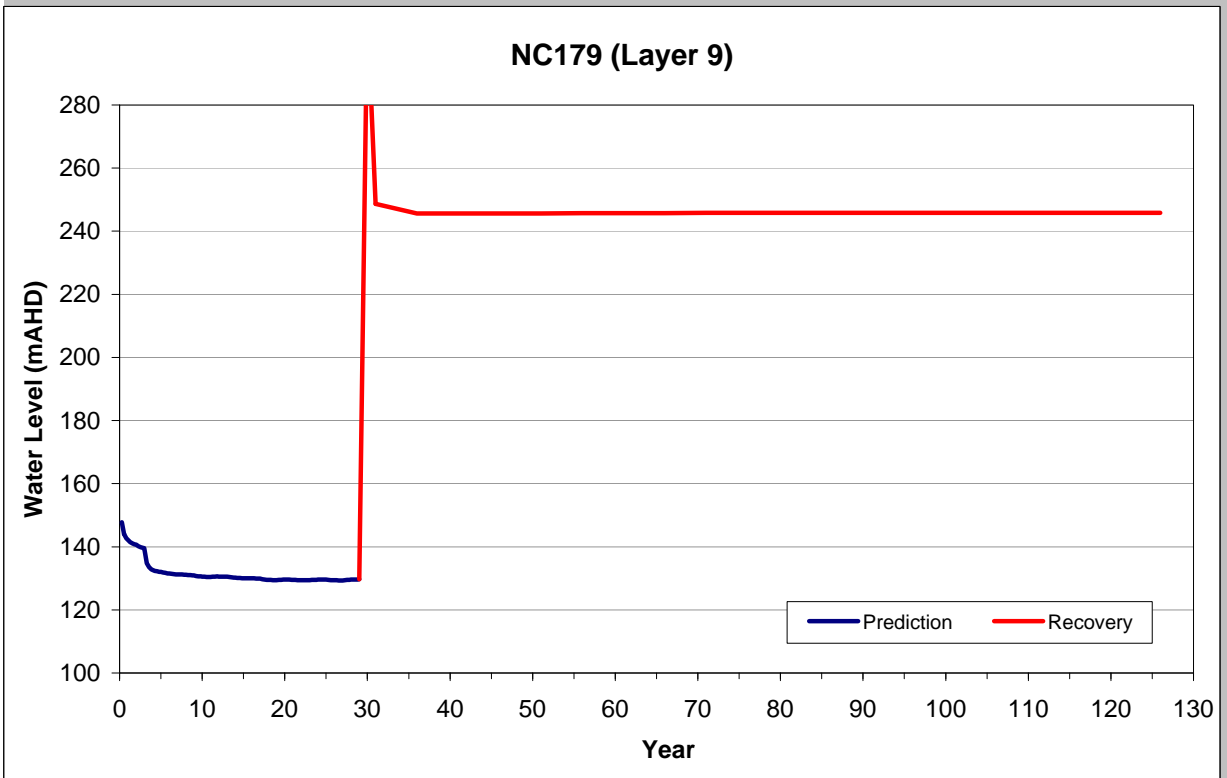
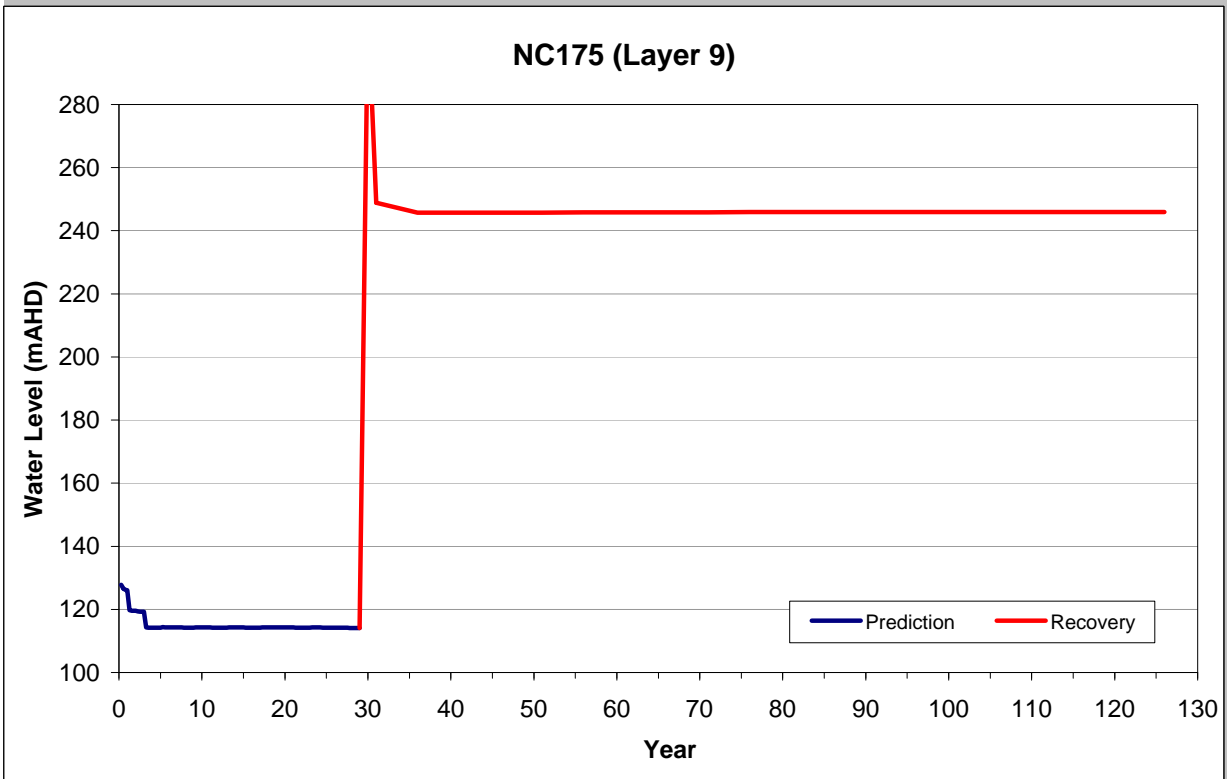


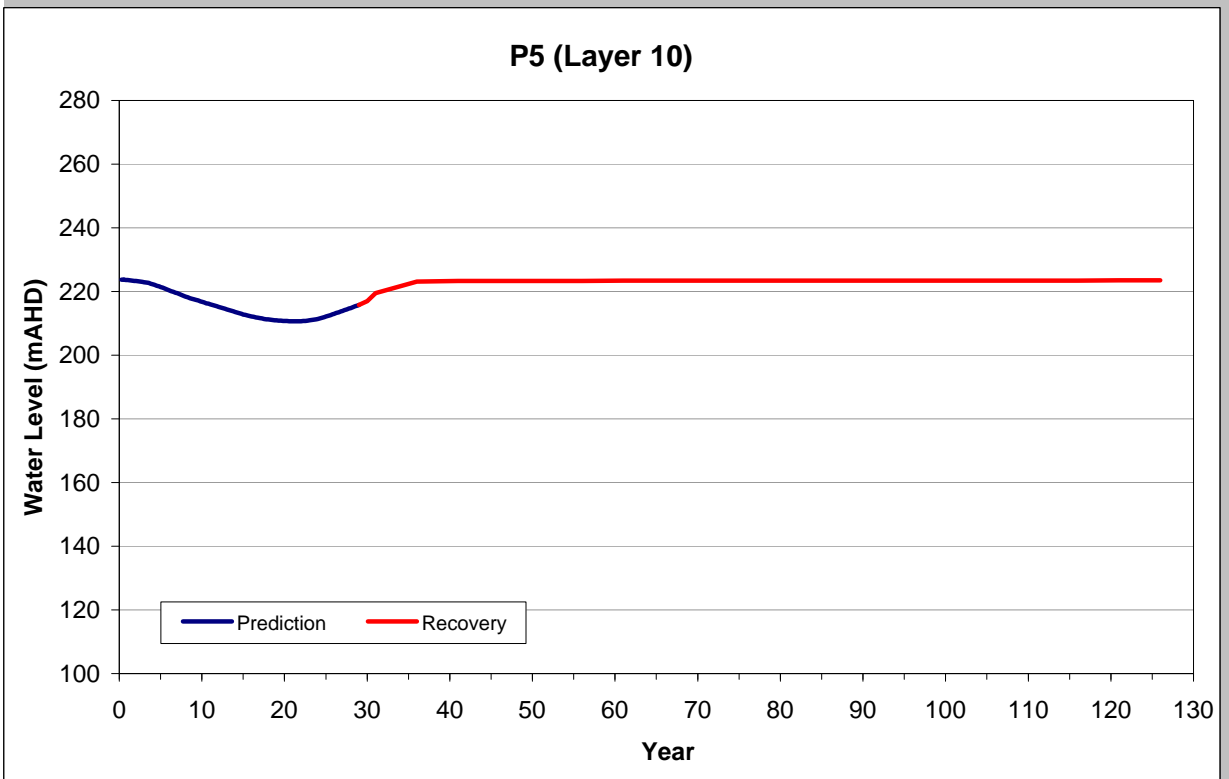
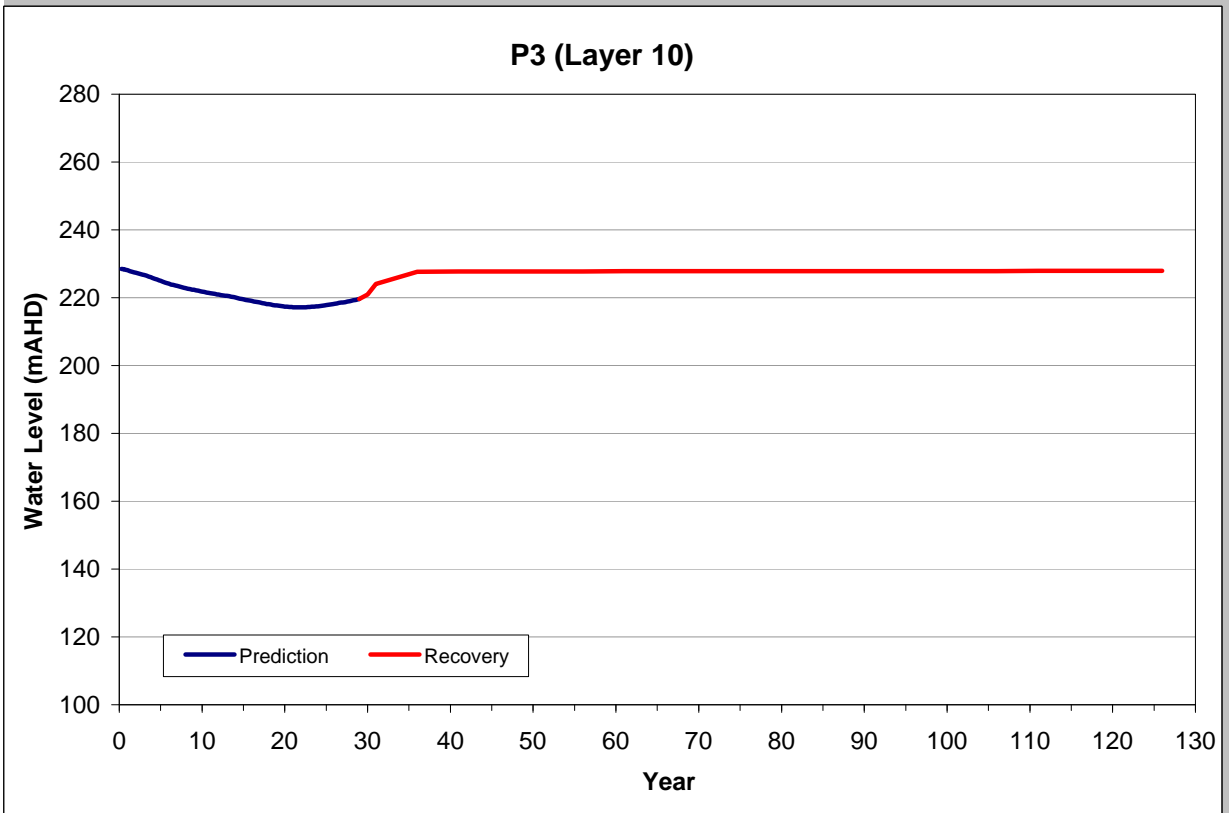


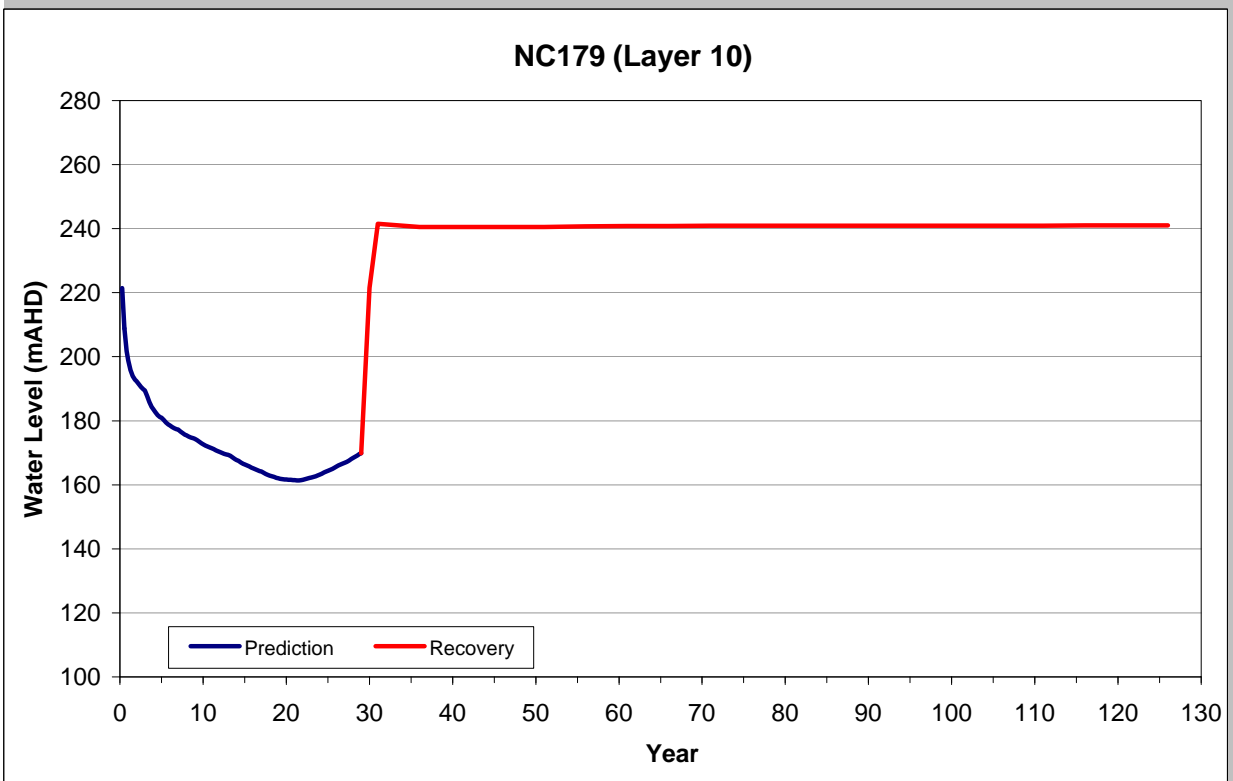
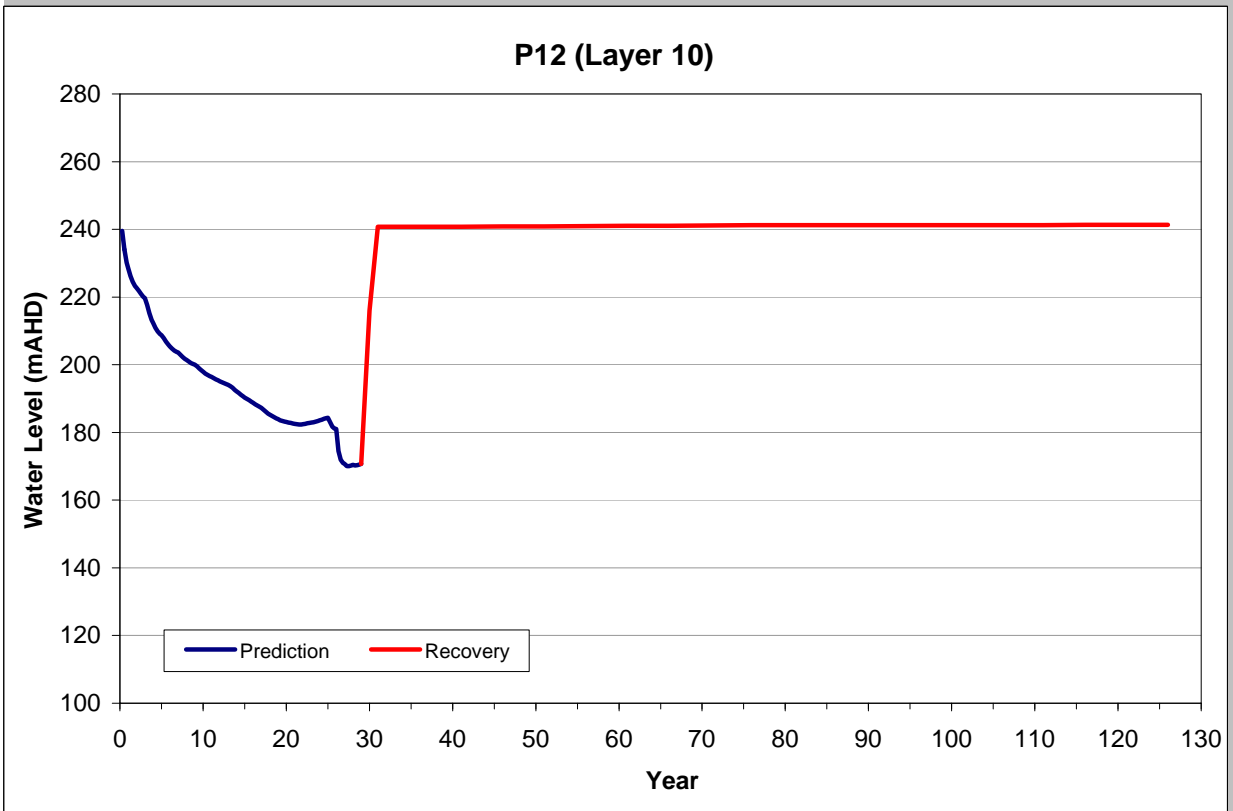




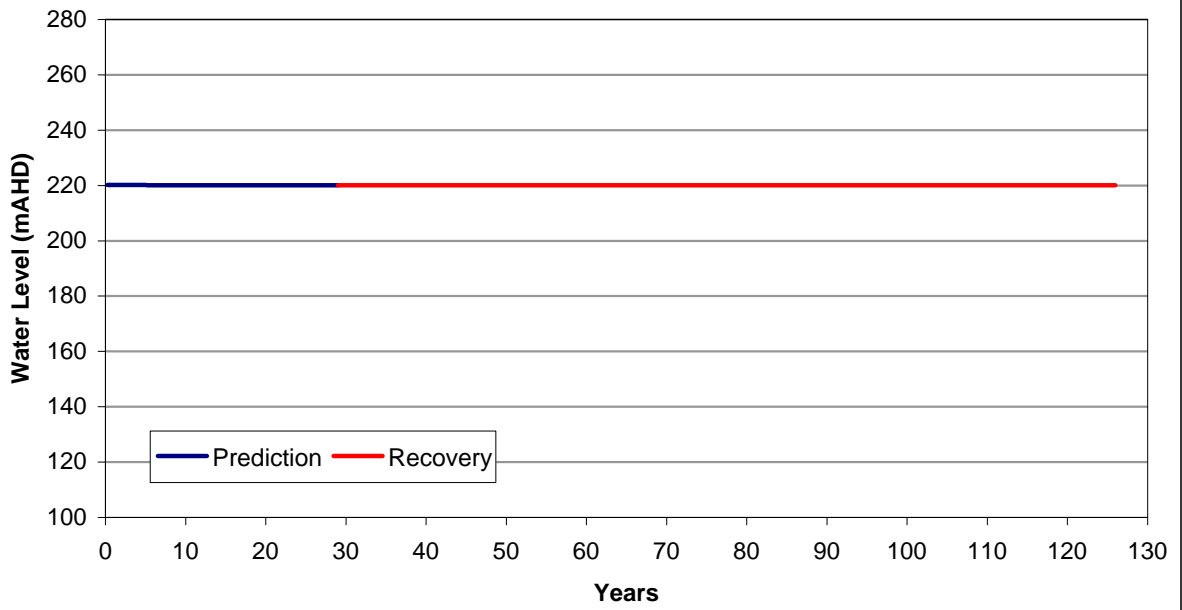




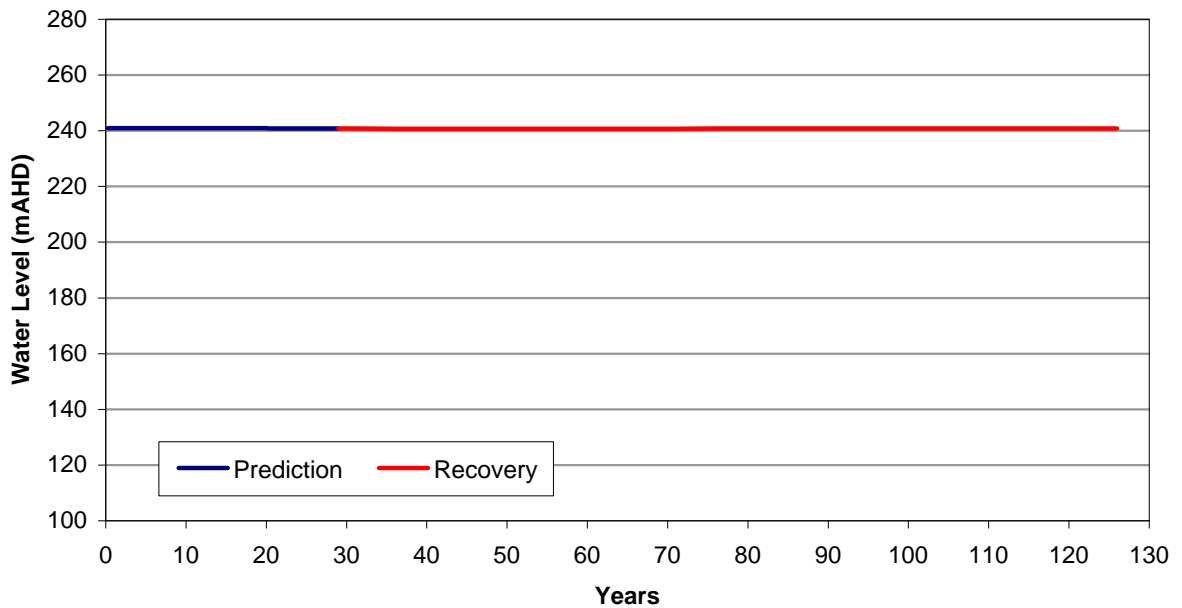




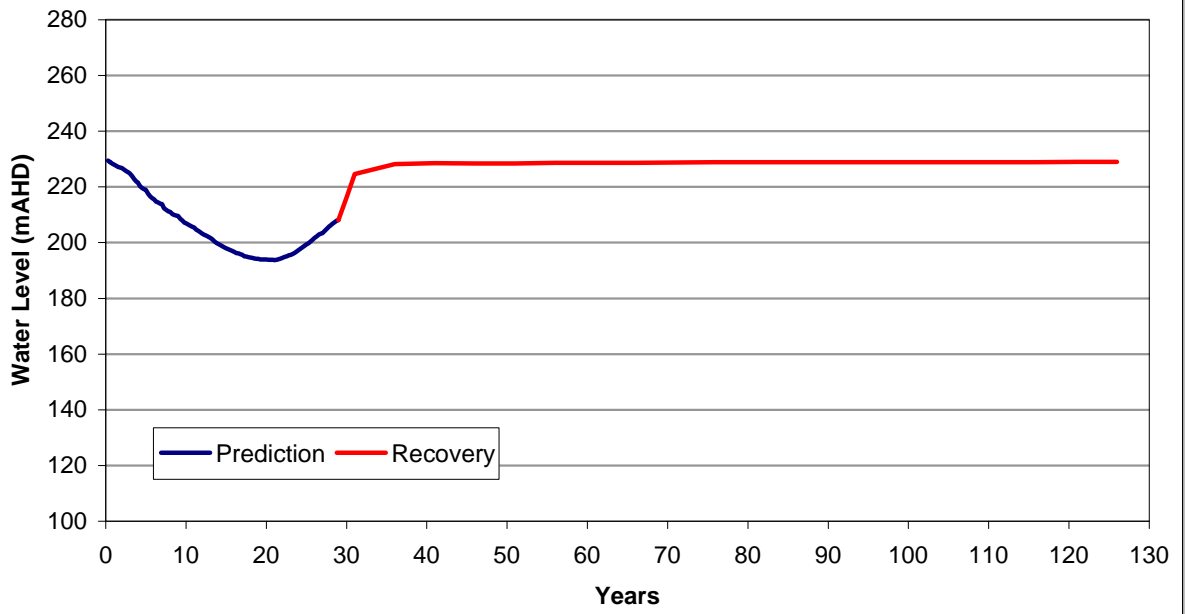
Dummy Bore 1 (Layer 1)



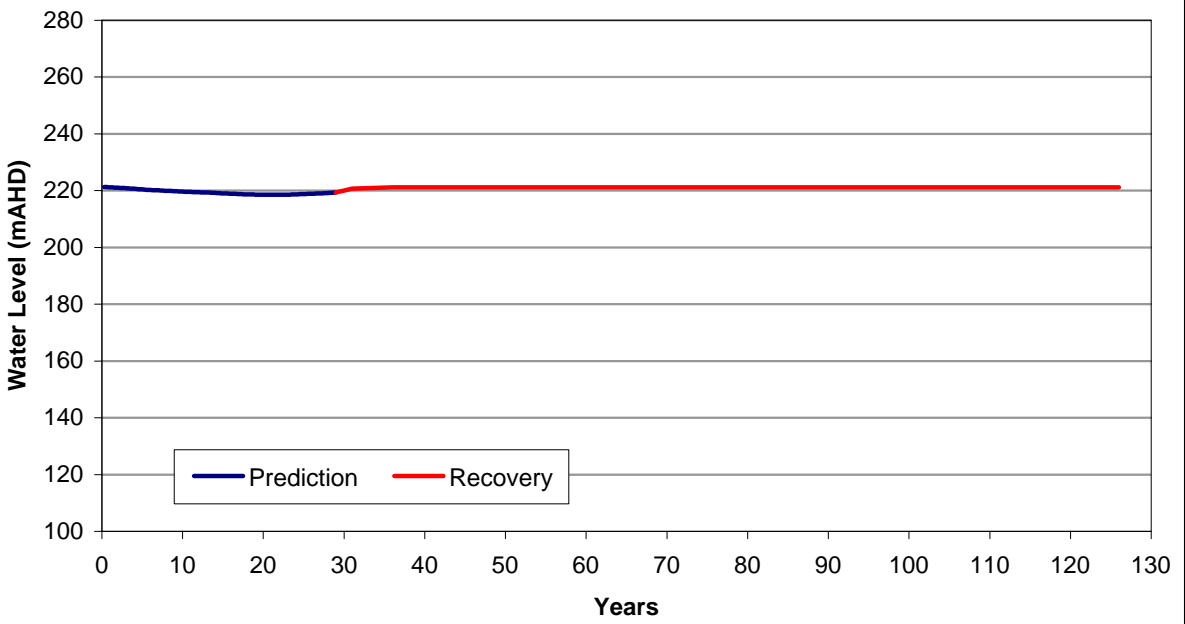
Dummy Bore 2 (Layer 1)



Dummy Bore 1 (Layer 11)



Dummy Bore 2 (Layer 11)



APPENDIX I

SHORT TERM PUMP TEST - CLAREMONT

Water and Environment

SHORT TERM PUMP TEST - CLAREMONT

Prepared for Narrabri Coal

Date of Issue 19 March 2009

Our Reference S28/B5/043



narrabri
water

SHORT TERM PUMP TEST - CLAREMONT

Prepared for Narrabri Coal

Date of Issue 19 March 2009

Our Reference S28/B5/043



SHORT TERM PUMP TEST - CLAREMONT

Date	Revision Description
Revision	

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Figure 1:	Location Diagram
Figure 2:	Location Diagram
Figure 3:	Pumping Phase Drawdown Data and Analysis
Figure 4:	Recovery Phase Drawdown Data and Analysis



1 INTRODUCTION

1.1 BACKGROUND

Aquaterra Pty Ltd was engaged by Narrabri Coal to undertake a pump test and groundwater assessment for groundwater bores screened within the Garrawilla Volcanics and the Narrabri Coal Project Site, NSW.

Drilling of coal resource delineation holes has revealed that groundwater bearing zones with airlift rates of up to 2-3 l/s have been encountered in fractured rock above longwall panel 1 (LW1). These areas correlate with sub cropping Garrawilla Volcanics and it is suspected that the volcanics at shallow levels have higher degree of fracturing to that observed at deeper levels due to unloading of overburden.

The concern is that fracturing caused by longwall mining related subsidence could connect the coal seam with the Garrawilla Volcanics at shallow depths of cover (that is over the first longwall panel proposed to be mined) and lead to high rates of inflow into underground workings for short periods.

There are a number of groundwater bores scattered over the local area, which are predominantly windmill driven pumps used for stock watering purposes, which are installed and screened within the volcanic unit. Pump tests carried out on one of these shallow bores resulted in very low yields, which resulted in the bore test being terminated. Consultation with property managers reveals this is typical of most of the bores in the vicinity, and the lower yields are adequate for stock water purposes. Even though some of these bores are in close proximity to exploration holes in which high yields were observed, they do not appear to connect with the high yielding fractures encountered in the exploration drilling.

This could indicate that the fissure systems are not well interconnected or extensive, and hence overall volumes of storage within the volcanics could be low. If this is the case, then boreholes that do not intersect the fissure system should only be capable of yielding at high rates for short periods of time.

One bore has shown reasonable yields and is located on the Claremont Property. It is used to supply a dam and is located approximately 100 m from a monitoring bore (P13) which is screened within the volcanic unit. The bores construction details and pump setting are unknown although it is understood that the bore is screened at 30 m and the pump set 20 meters below ground level (mbgl). The pump is powered at the surface by a small diesel engine and is capable of yielding approximately 1 L/s.

A pump test was undertaken on this bore using the installed infrastructure to assess the basic hydraulic characteristics to assess the levels of storage that may exist within the fractured rock aquifer.

1.2 SCOPE OF WORK

The scope of work for this study comprised:

- ▼ Undertaking a 4 hour pump test on the bore with drawdown monitored during pumping and for a maximum of 4 hours during recovery.
- ▼ Collection of 1 water sample at 4 hours pumping time and submittal to laboratory for analysis of dissolved constituents.
 - ▼ Analysis and reporting comprising an assessment of
 - ▼ pump test results;
 - ▼ water quality against various criteria; and
 - ▼ impacts of bore operation on surrounding groundwater users and the environment.



2 PHYSICAL SETTING

2.1 TOPOGRAPHICAL SETTING

The Narrabri Project is located approximately 30 km southeast of Narrabri (Figure 1). The Project Site occupies the northern half of EL 6243, covers approximately 5 900 hectares and is located primarily in cleared grazing land (Figure 1.2).

The topography of the Project Site generally slopes to the east and northeast from over 300 m AHD in the southwest of the site to 270 m AHD in the east. While topography generally dips to the east and northeast, surface contours exhibit gentle undulations with surface drainage pathways occurring across the site in a north-easterly direction.

2.2 CLIMATE

The climate of the region is cool to temperate with seasonal climatic variations ranging from hot summers to cool winters. The average daily maximum temperature ranges from 35.3 °C in January to 17.0 °C in July.

2.2.1 RAINFALL

Rainfall data is available from Narrabri Airport (Station 54038).

Table 2.1 Long-term Average (LTA) Monthly Rainfall at Narrabri Airport (mm/day)

Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Average
2.18	2.52	1.05	0.91	0.56	2.41	0.92	0.83	0.99	1.23	2.18	3.46	1.60

Source: Bureau of Meteorology (2008)

2.2.2 EVAPORATION

The nearest meteorological station with long term evaporation data is Tamworth Airport (Station 55054).

Table 2: Long-term Average Monthly Potential Evaporation Rates at Tamworth Airport (mm/day)

Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Average
8.6	8.1	6.9	4.6	2.9	2.0	2.1	3.0	4.4	6.0	7.6	8.7	5.4

Source: Bureau of Meteorology (2008)



3 PUMP TEST ANALYSIS

3.1 FIELD ACTIVITIES

A 4-hour constant discharge pumping test at a rate of 0.95 L/s was carried out in February 2009. A 10m discharge pipe was run discharge groundwater into a drainage channel to avoid potential recharge to the aquifer during the test. Uniform clay at surface is expected to have sealed the aquifer from recharge during this test. Water level drawdown was measured at appropriate time intervals using an electronic water level meter during the 4 hours of pumping and during the recovery.

One water sample was collected at the end of pumping time for submission to a NATA registered laboratory (Australian Laboratory Services Pty Ltd) for analysis. The sample was analysed for major ions (sodium, calcium, magnesium, potassium, chloride, and sulphate), alkalinity, dissolved metals (arsenic, cadmium, chromium, copper, nickel, lead, mercury, and zinc), pH, electrical conductivity and total dissolved solids to assess the character of the groundwater.

The location of the Claremont property pump bore and P13 are shown in Figure 2.

3.2 PUMP TEST RESULTS

Drawdown data have been analysed using the Cooper-Jacob straight line method (Cooper and Jacob, 1946). Pumping and recovery phase drawdown data, together with the data analysis, are shown in Figures 3 and 4 respectively.

The static water level in the bore prior to the pump test was 5.24 metres below the top of casing (mbtoc). The water level dropped 1.5m in the first 2 minutes of pumping. After 2 minutes, the rate of water level decline decreased markedly. By the end of the 4 hour pumping phase the water level had declined to 6.77 mbgl (a drawdown of 1.77 metres). Following pumping, the water level recovered to 90% of the pre-pumping level in 2 minutes. It is assessed that no hydraulic barrier boundaries were intersected by the drawdown cone during the test.

The results of the pump test analyses are summarised in Table 3. The calculated hydraulic conductivity is an average value calculated using an assumed aquifer thickness of 37m (the interval between the lowest water level during the test to the base of the bore) and does not take into account vertical flow components (which are likely to be small). The average transmissivity of the volcanics is assessed to be around 75 m²/day. This result indicates a higher permeability for the volcanic unit than normally encountered and suggests the influence of structural features.

No drawdown was observed in the nearest monitoring bore (P13). This is unusual given its proximity to the pumped borehole (approximately 100m) and indicates that almost all of the water abstracted during pumping was being rapidly taken from a fracture system that was not connected to the observation borehole.

Table 3: Summary of Hydraulic Testing

Item	Transmissivity (m ² /day)
Pumping Phase	100
Recovery Phase	75

3.3 LABORATORY ANALYTICAL RESULTS

Laboratory analytical results for water samples collected at end of pumping is summarised in Table 4. Species concentrations have been compared to ANZECC guidelines where concentrations of tested analytes are below relevant ANZECC guidelines except in the case of zinc for freshwater aquatic ecosystems. The groundwater is essentially pH neutral and is



dominated by bicarbonate and sodium ions. The overall water quality is good and the pH is within the ANZECC guideline range.

Table 4: Water Sample Laboratory Analytical Results

Sample ID	INVESTIGATION LEVELS				Date of Sampling
	Freshwater	Irrigation	Recreation		
Geological Origin					24/02/2009
Date of Sampling					
GENERAL PARAMETERS					
pH		6-8.5 ²	5.0-9.0 ³		7.26
Electrical Conductivity					1080
MAJOR CATIONS					
Calcium					65
Magnesium					54
Sodium			300 ³		97
Potassium					1
MAJOR ANIONS					
Sulphate as SO ₄ ²⁻			400 ³		38
Chloride			400 ³		126
Bicarbonate as CaCO ₃					324
HEAVY METALS					
Arsenic	0.024 ¹	0.1 ²	0.05 ³		<0.01
Cadmium	0.0002 ¹	0.01 ²	0.005 ³		<0.001
Chromium	0.0033 ^{1,5}	0.1 ²	0.05 ³		<0.001
Copper	0.0014 ¹	0.2 ²	1 ³		0.001
Lead	0.0034 ¹	2 ²	0.05 ³		<0.001
Nickel	0.011 ¹	0.2 ²	0.1 ³		0.004
Zinc	0.008 ¹	2 ²	5 ³		0.011
Mercury	0.0006 ¹	0.002 ²	0.001 ³		<0.0001

NOTES:

- | |
|-----|
| 100 |
|-----|

 Concentration exceeds the ANZECC Guidelines for protection of 95% species
- | |
|-----|
| 100 |
|-----|

 Concentration exceeds the ANZECC Guidelines for irrigation (long term)
- | |
|-----|
| 100 |
|-----|

 Concentration exceeds the ANZECC Guidelines for recreation purposes (secondary)
- Based on ANZECC (2000) trigger values for protection of 95% of fresh water species
- ² Based on ANZECC (2000) Long term trigger values for irrigation
- Based on ANZECC (2000) Water quality guidelines for recreational purposes (secondary)
- ³

4



3.4 LIMITING BORE YIELD

The limiting bore yield is the yield the bore is able to supply over a specified time frame without dewatering the bore. It is an operational concept related to bore completion and intended operation, and does not consider drawdown in the surrounding aquifer. A broad estimate of the limiting bore yield can be made using the method presented in Hazel (1973). This method adopts the following equation:

$$QL = QT \text{ sw/sp}$$

Where (adopted values in brackets):

- ▼ QL is the limiting yield
- ▼ QT is the pump test discharge rate (0.95L/s)
- ▼ sw is the available drawdown (20m)
- ▼ sp is the drawdown in the bore measured during the pump test, extrapolated to an arbitrary time considered appropriate for the intended bore pumping schedule (365 days).

This calculation is approximate only, and makes no allowance for intersection of barrier (or recharge) boundaries. A time period of 365 days is adopted taking into account the intended use of the bore (see Hazel, 1973). The drawdown at 365 days extrapolated from pump test results is around 2m. Thus a broad estimate of the limiting bore yield is around 10L/s.

However, given the fact that water is being drawn from a fracture system of unknown size and extent, this may not be representative of the actual yield if abstraction continues for more than a few hours



4 CONCLUSIONS AND RECOMMENDATIONS

The aquifer was not highly stressed during the short pump test at the Claremont property, and the borehole is capable of yielding a significantly higher volume of groundwater than was extracted during this pump test. The lack of response in the nearby monitoring standpipe and the low level of drawdown in the bore during the test indicate that there is a level of storage within the fractured rock aquifer that was not clearly defined by a test of this size.

It is important to understand the level of storage that is contained within the fractured rock aquifer in order to understand the risk that is posed to underground mine workings. It is therefore recommended that the bore be retested at a higher rate of at least 3-5 l/s for a longer period. Ideally this should be at least 12 hours, although this does depend on well performance during the test. A test of this nature is recommended in order to better understand the extent and interconnectivity of the fractured rock aquifer and to better understand the risk this represents to LW1 in terms of elevated groundwater inflows caused by mining disturbance.

Because of the duration and size of the test, the calculation of bore yield limits (10l/s) is an approximation only and, given that boundary affects are likely to be experienced, constant pumping rates of this magnitude are unlikely to be sustainable. Personal communications with the property manager suggests that rates of 5 l/s lead to maximum drawdown within a 12 hour period.

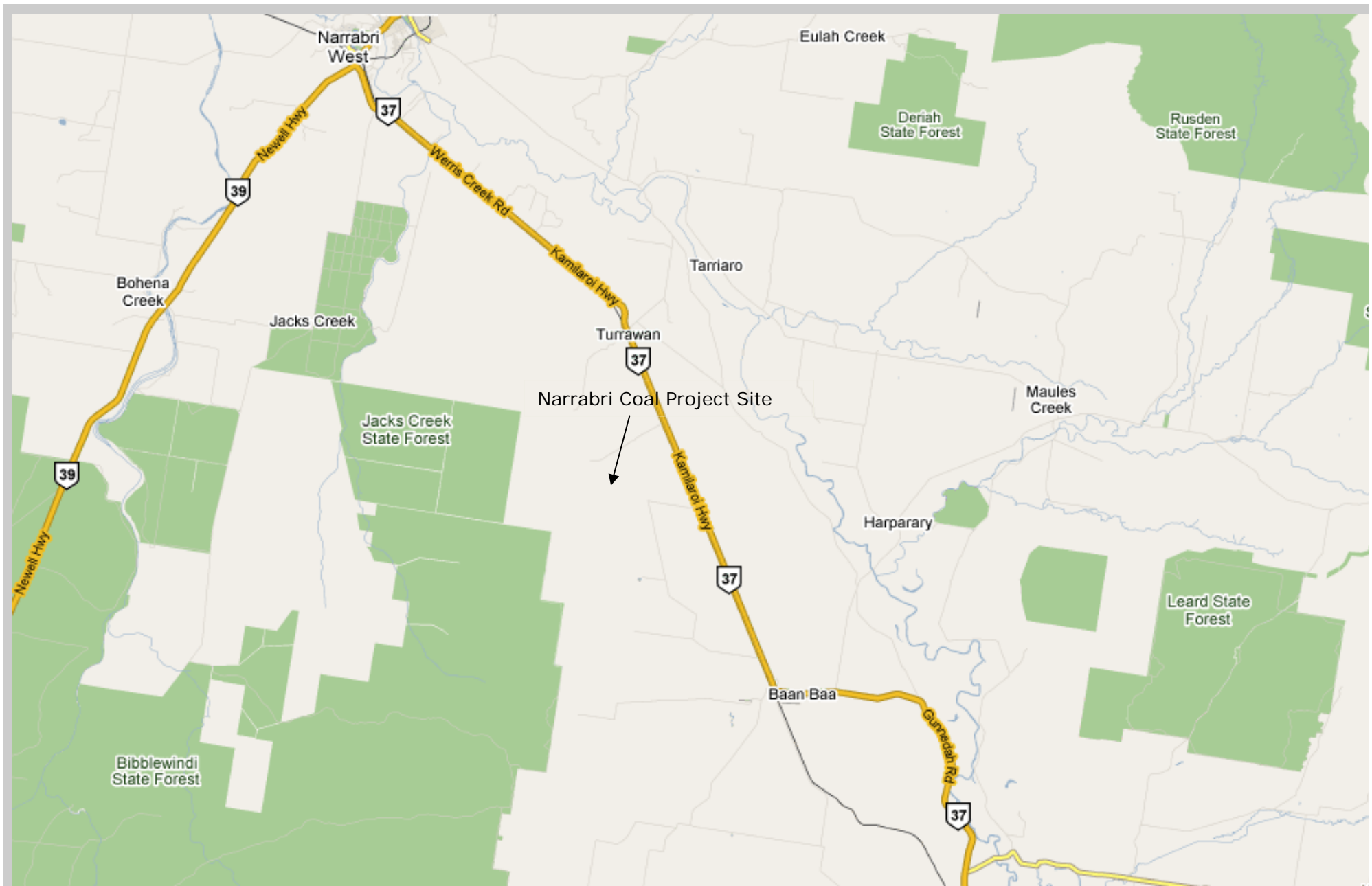
Laboratory analytical results indicate that water quality is very good with a low overall TDS of 564 mg/l. The groundwater signature was dominated by high proportion of bicarbonate ions may indicate that this is a recharge area. The low overall level of dissolved ions also suggests that a significant proportion of recharge is of rainfall origin.

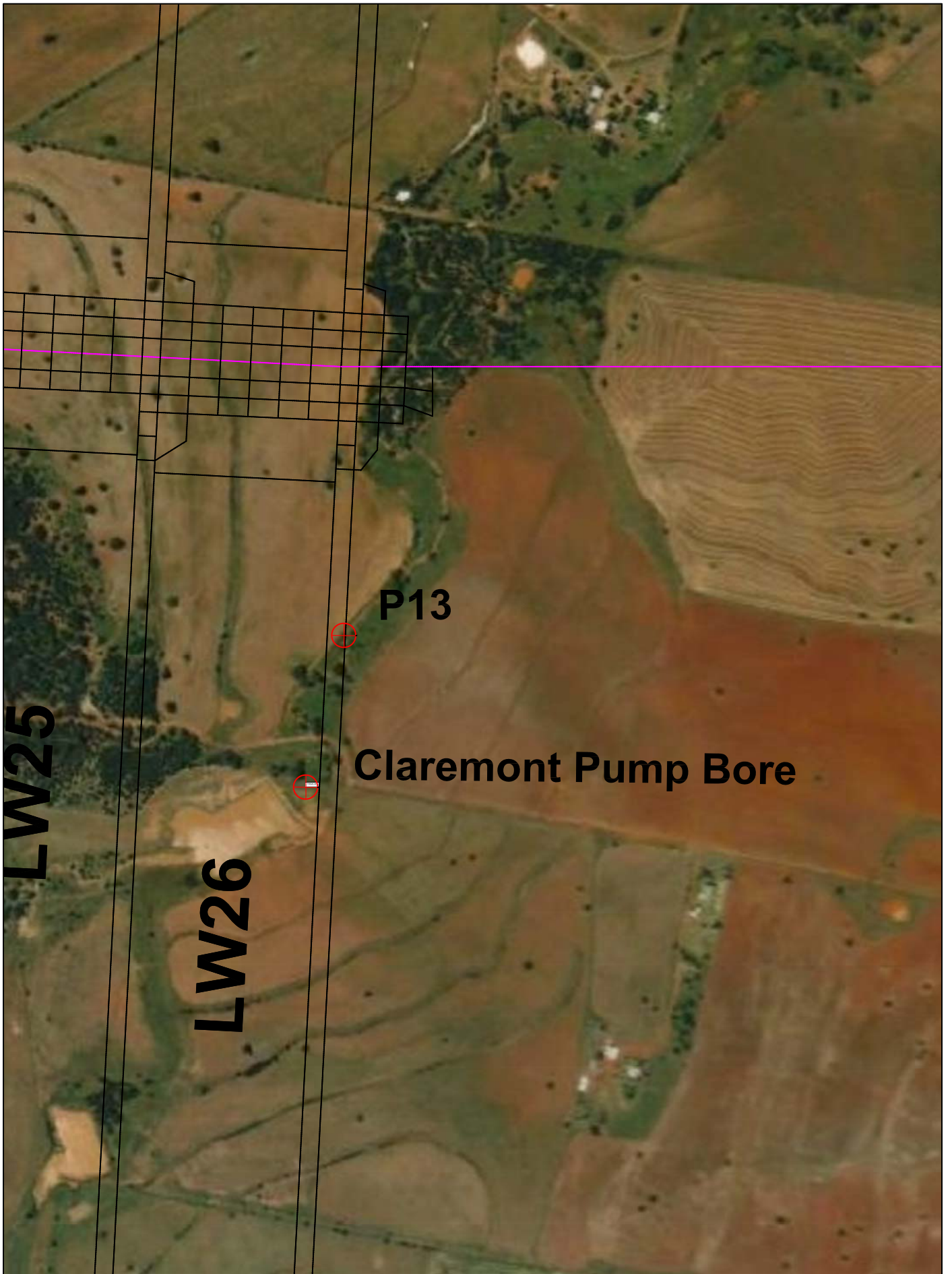


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Kruseman GP and de Ridder NA. 1994. Analysis and Evaluation of Pumping Test Data. Second Edition, International Institute for Land Reclamation and Improvement, Publication 47.

Water Resources Consulting Services, 1997. Bicarbonate Occurrence in Groundwater in the Baan Baa Area, NSW

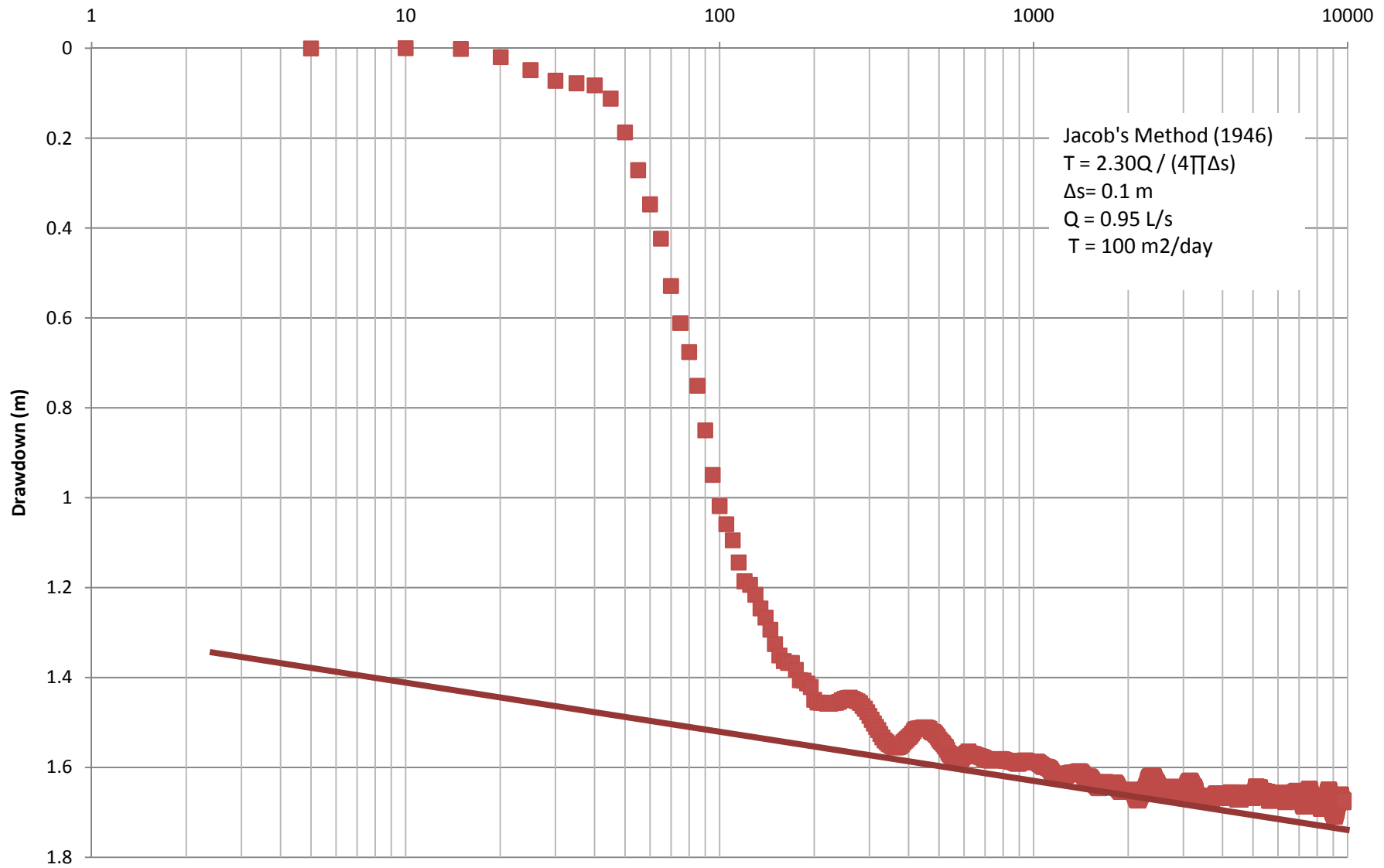




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Initials:	Job No: S28 / B5	
Drawing No: S28-B5/02	Rev: A	
aquaterra		Figure 2

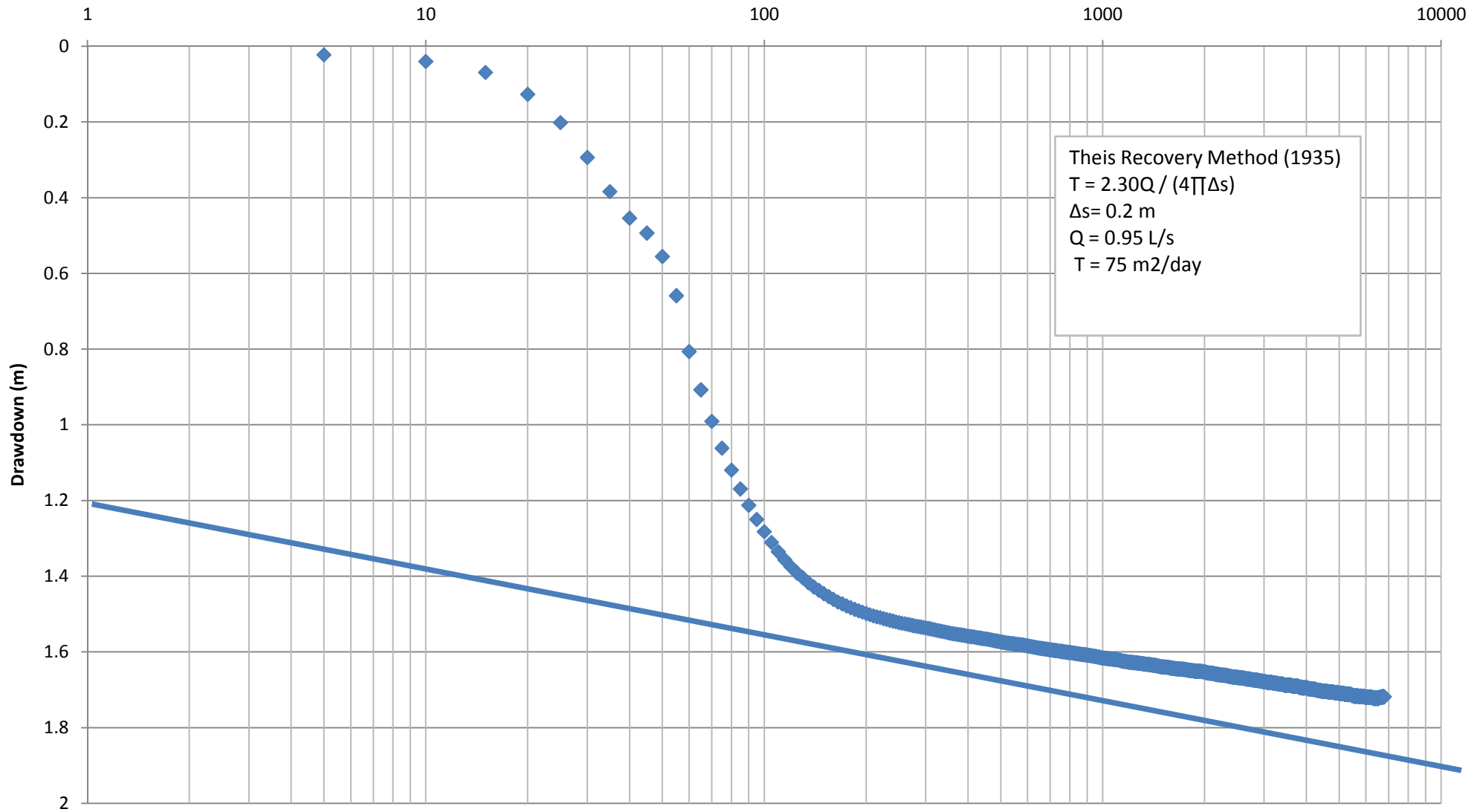
Drawdown

Time since p (seconds)



Recovery

Time t/t' (seconds)





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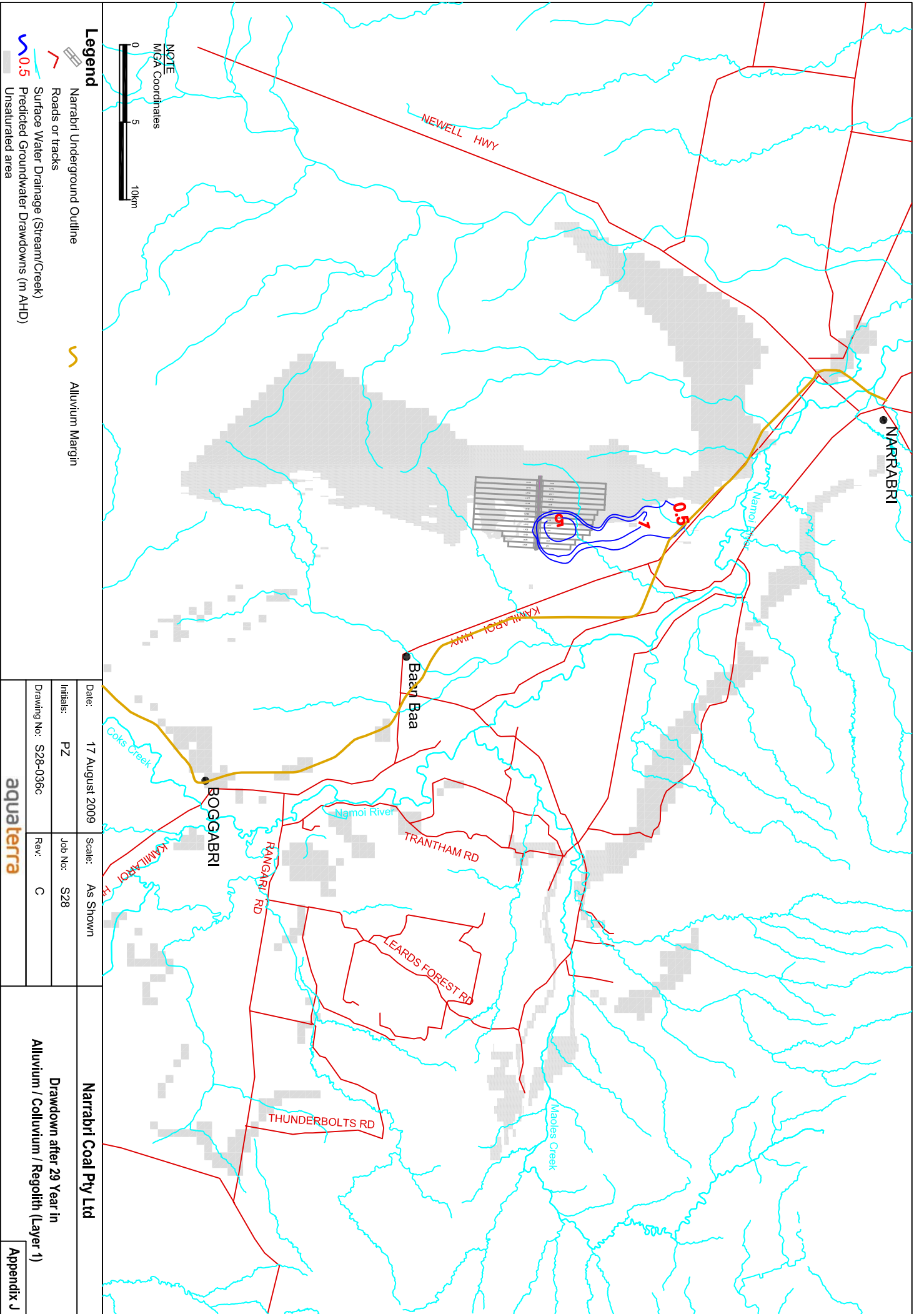
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Water and Environment

APPENDIX J

NARRABRI DRAWDOWN AT THE END OF MINING



Date: 17 August 2009

Initials: PZ

Drawing No: S28-036c

Scale: As Shown

Job No: S28

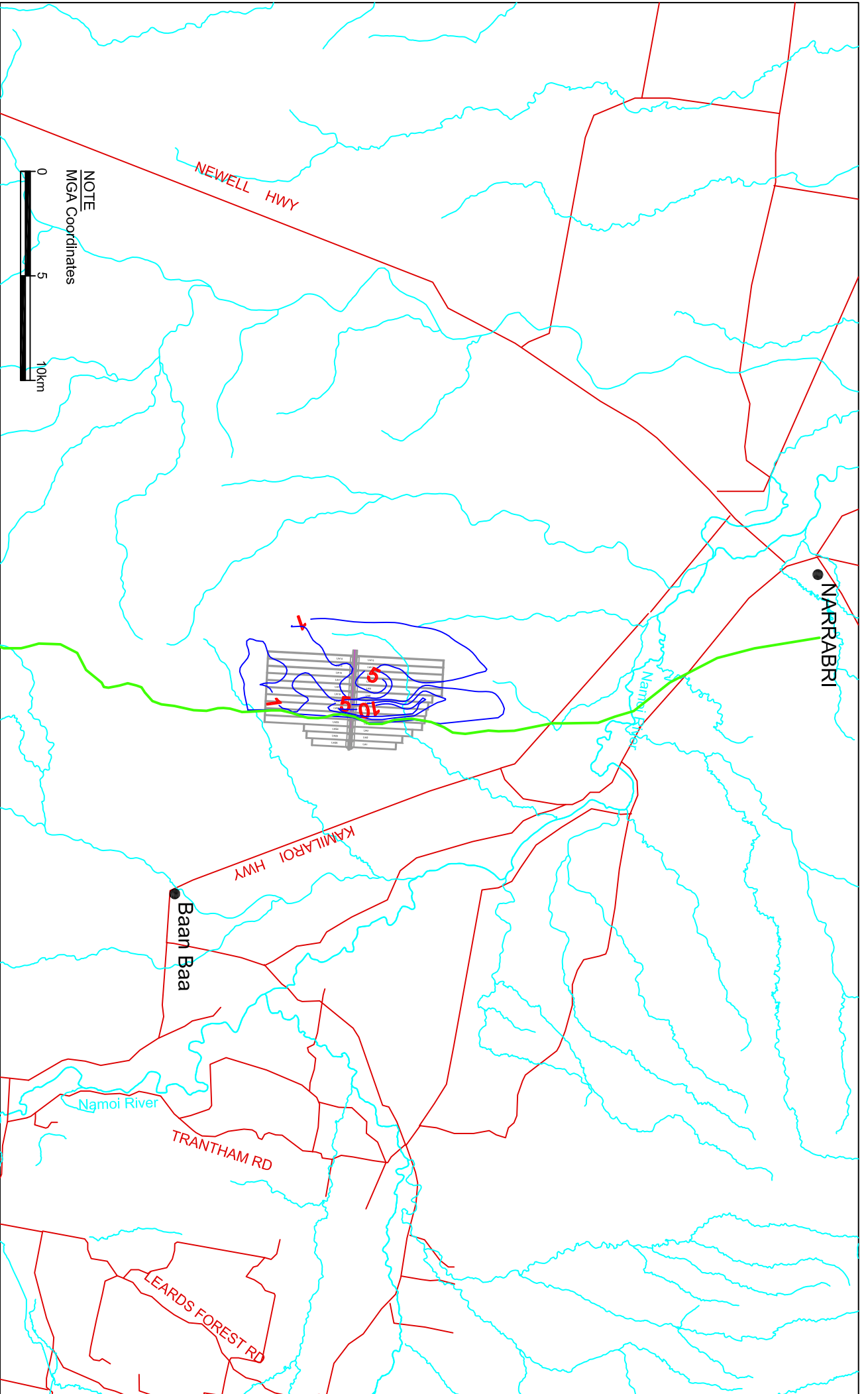
Rev: C

Narrabri Coal Pty Ltd

Drawdown after 29 Year in Alluvium / Colluvium / Regolith (Layer 1)

aquaterra

Appendix J



Legend

- Narrabri Underground Outline
- Roads or tracks
- Surface Water Drainage (Stream/Creek)
- Piliiga Sandstone Sub Crop Margin
- 0.5 Predicted Groundwater Drawdowns (m AHD)
- Unsaturated area

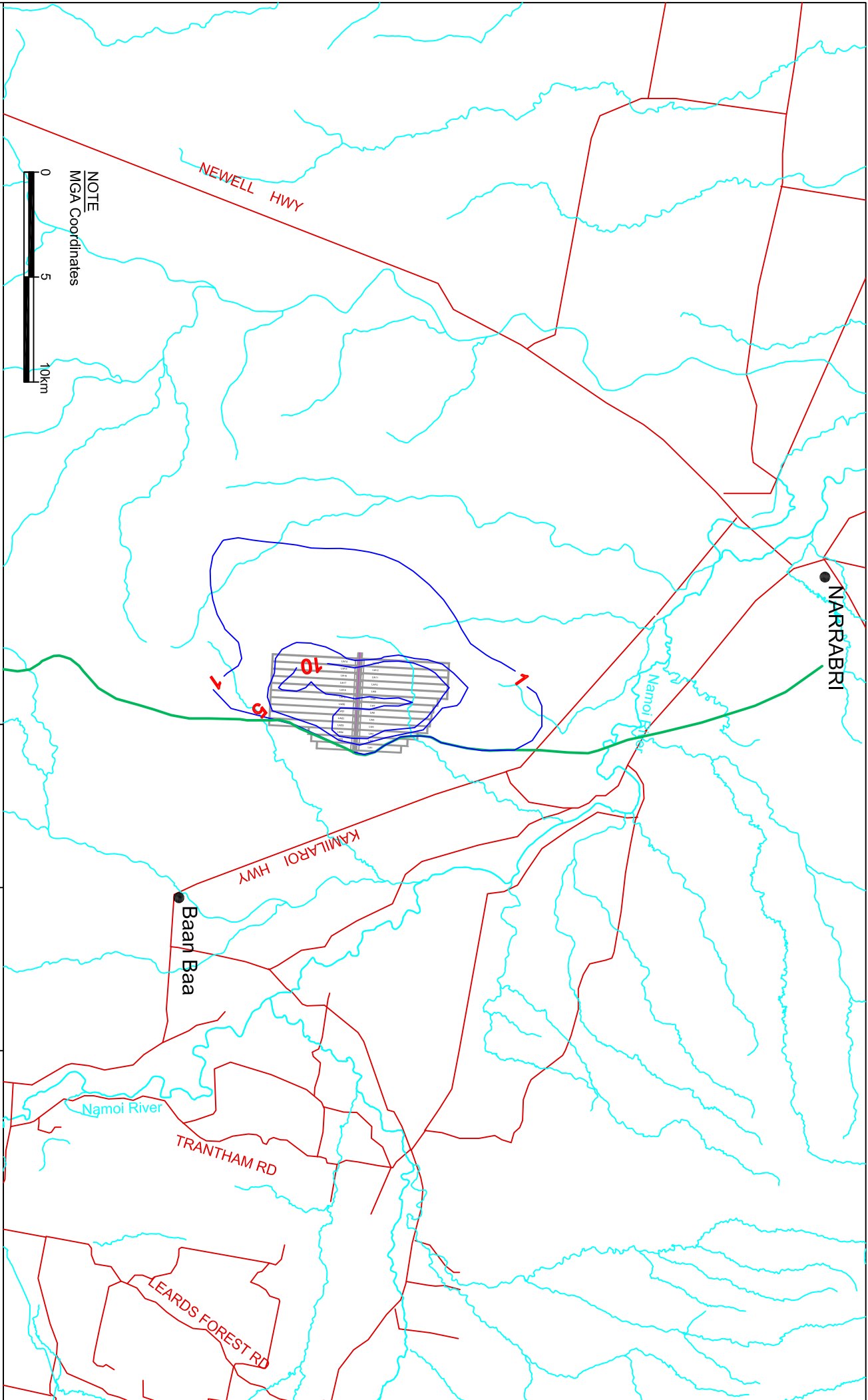
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Initials:	PZ	Job No:	SZ8
Drawing No:	S28-080	Rev:	0

Narrabri Coal Pty Ltd







Drawdown after 29 Year in
Piliiga Sandstone (Layer 2)

aquaterra

Appendix J



Legend

-  Narrabri Underground Outline
-  Roads or tracks
-  Surface Water Drainage (Stream/Creek)
-  0.5 Predicted Groundwater Drawdowns (m AHD)
-  Purlawaugh Formation Sub Crop Margin
-  Unsaturated area

NOTE
MGA Coordinates



Date: 18 August 2009

Initials: PZ

Drawing No: S28-081

Scale: As Shown

Job No: S28

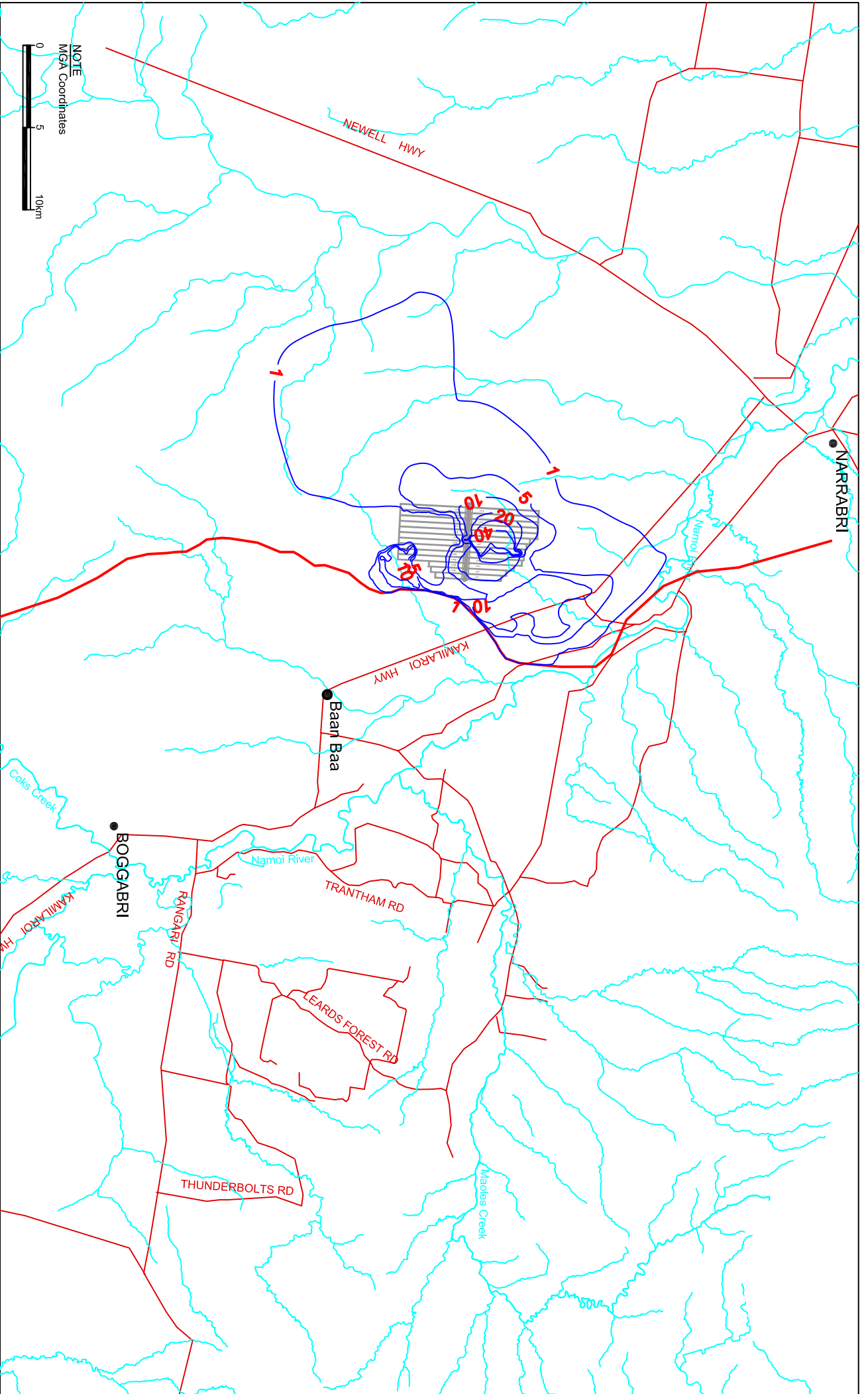
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Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Purlawaugh Formation (Layer 3)

aquaterra




Appendix J



NOTE
MGA Coordinates



Legend

-  Narrabri Underground Outline
-  Roads or tracks
-  Surface Water Drainage (Stream/Creek)
-  Garrawilla Volcanics Sub Crop Margin
-  0.5 Predicted Groundwater Drawdowns (m AHD)

Date: 12 August 2009

Scale: As Shown

Narrabri Coal Pty Ltd

Initials: PZ

Job No: S28

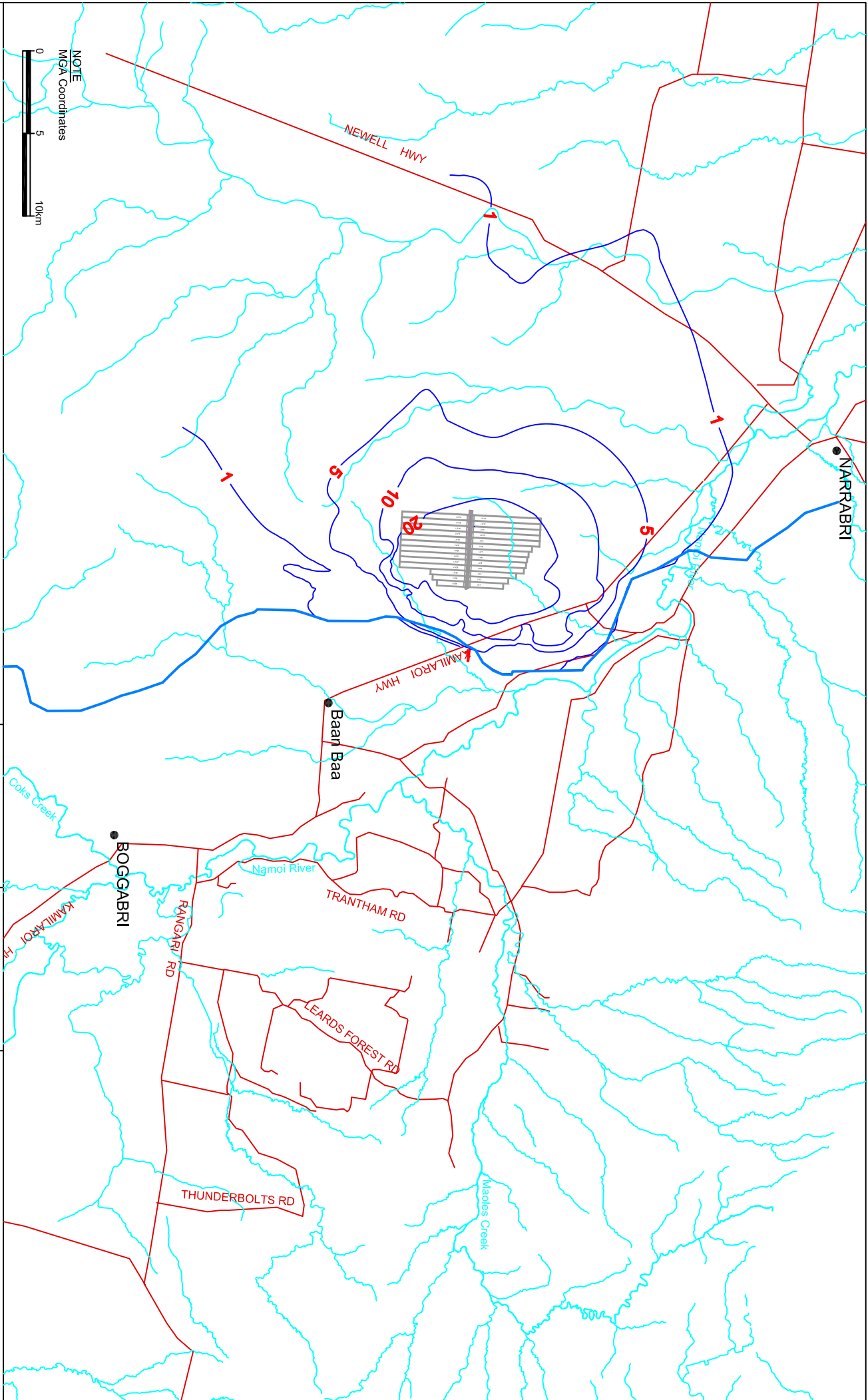
Drawdown after 29 Year in
Garrawilla Volcanics (Layer 4)

Drawing No: S28-045g

Rev: G

aquaterra

Appendix J



Legend

-  Narrabri Underground Outline
-  Roads or Tracks
-  Surface Water Drainage (Stream/Creek)
-  Predicted Groundwater Drawdowns (m AHD)
-  Napperby Formation Boundary

NOTE
MGA Coordinates
0 5 10km

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Scale: As Shown

Initials: PZ
Job No: S28

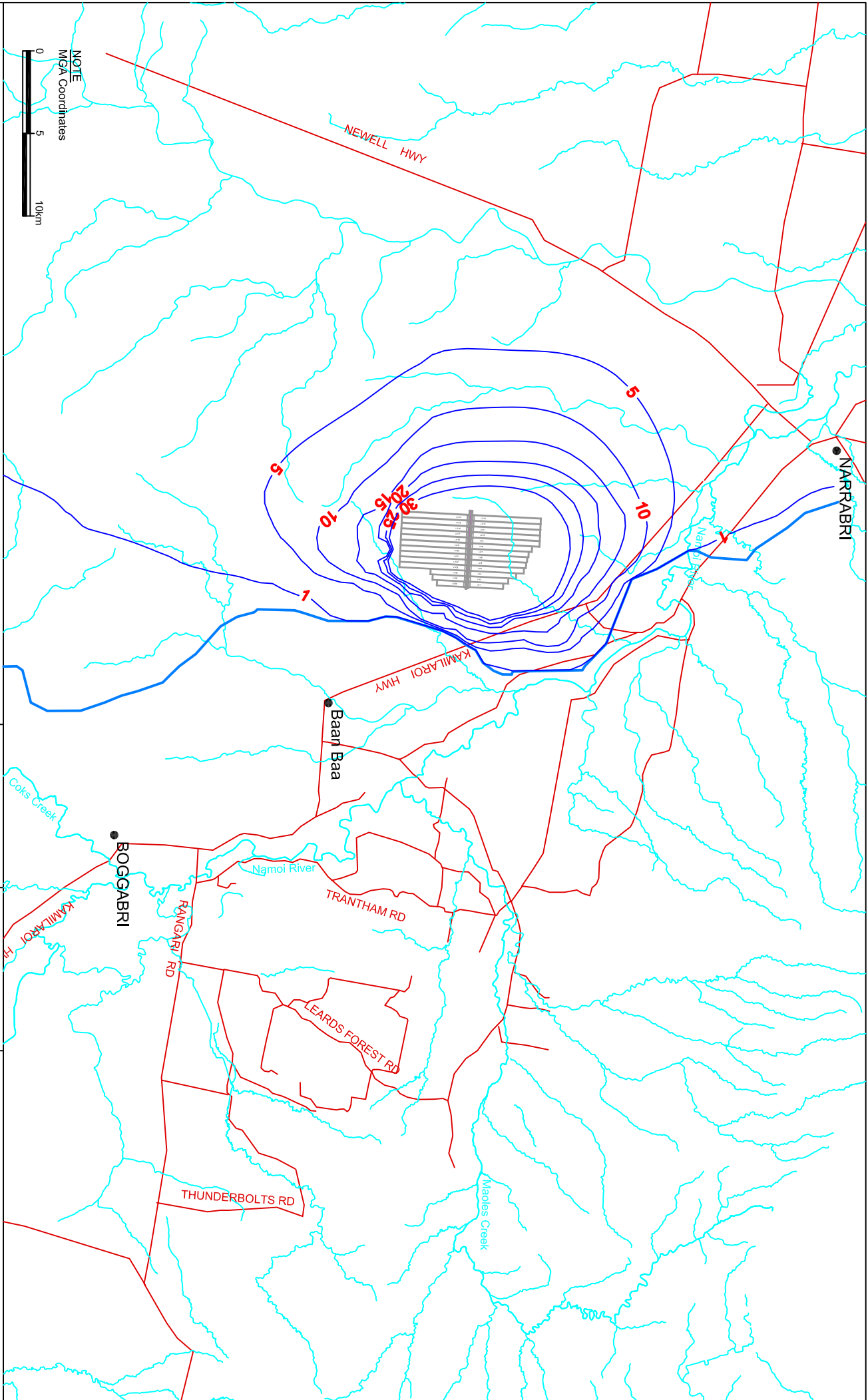
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Rev: C

Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Napperby Formation (Layer 5)

aquaterra

Appendix J



NOTE
MGA Coordinates



Legend

- Narrabri Underground Outline
- Roads or Tracks
- Surface Water Drainage (Stream/Creek)
- Predicted Groundwater Drawdowns (m AHD)
- Napperby Formation Boundary

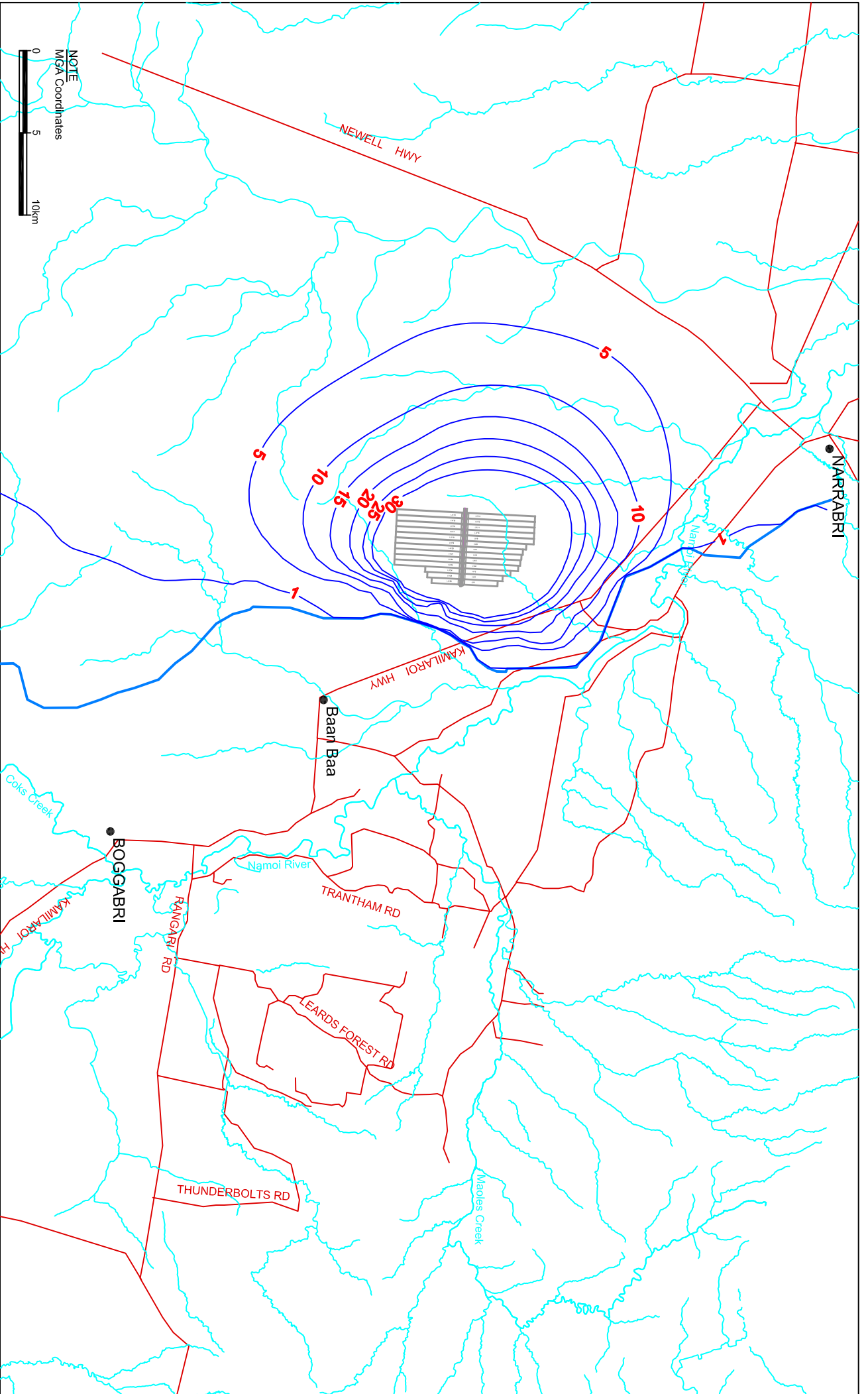
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Drawing No:	S28-082	Rev:	0

Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Napperby Formation (Layer 6)

Appendix J


aquaterra



NOTE
MGA Coordinates



Legend

-  Narrabri Underground Outline
-  Roads or Tracks
-  Surface Water Drainage (Stream/Creek)
-  Predicted Groundwater Drawdowns (m AHD)
-  Napperby Formation Boundary

Date: 17 August 2009

Initials: PZ

Drawing No: S28-082

Scale: As Shown

Job No: S28

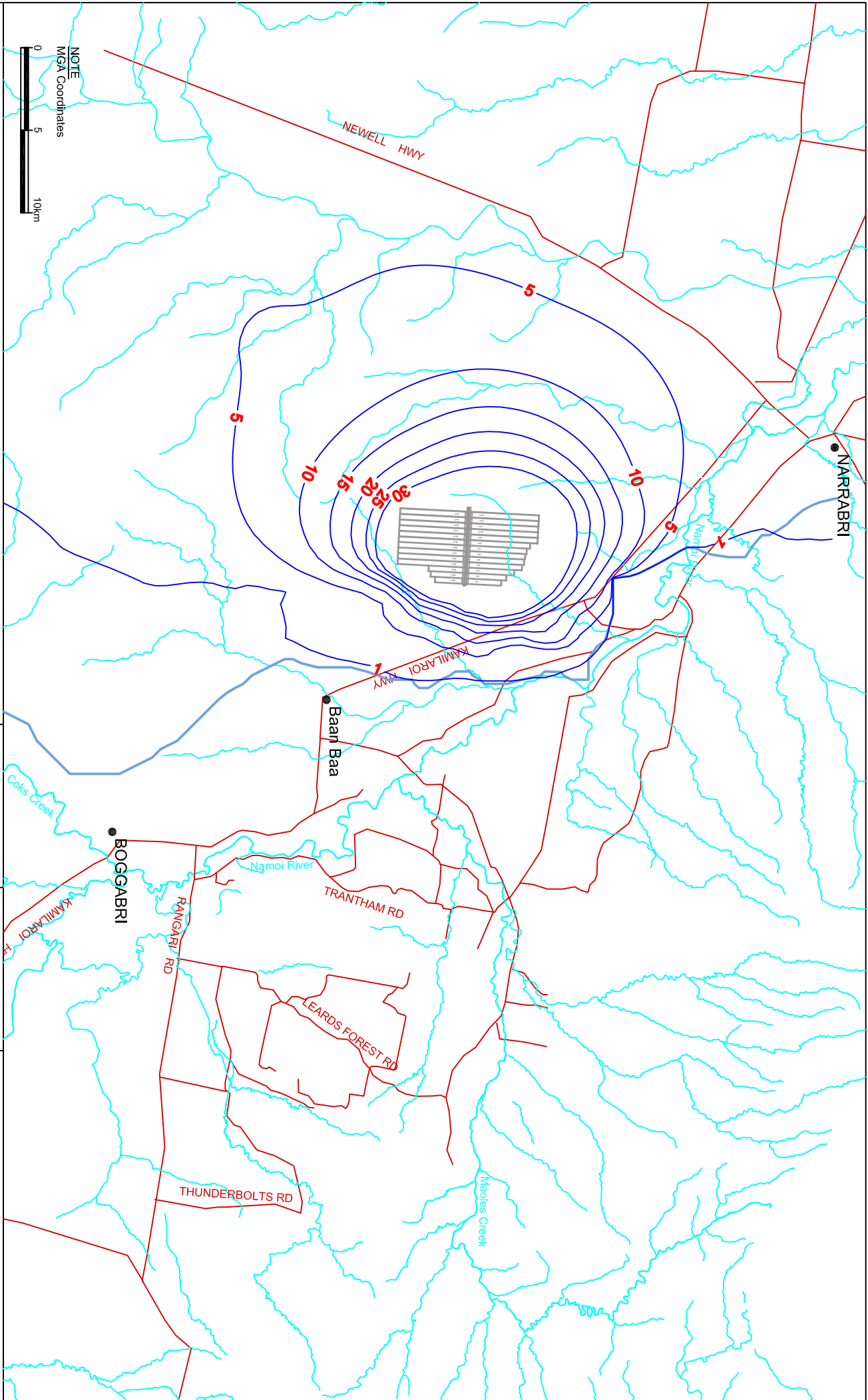
Rev: 0

Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Napperby Formation (Layer 7)

aquaterra

Appendix J



Legend

-  Narrabri Underground Outline
-  Roads or tracks
-  Surface Water Drainage (Stream/Creek)
-  Predicted Groundwater Drawdowns (m AHD)
-  Digby Formation Sub Crop Margin

NOTE
MGA Coordinates



Date: 14 August 2009

Initials: PZ

Drawing No: S28-084

Scale: As Shown

Job No: S28

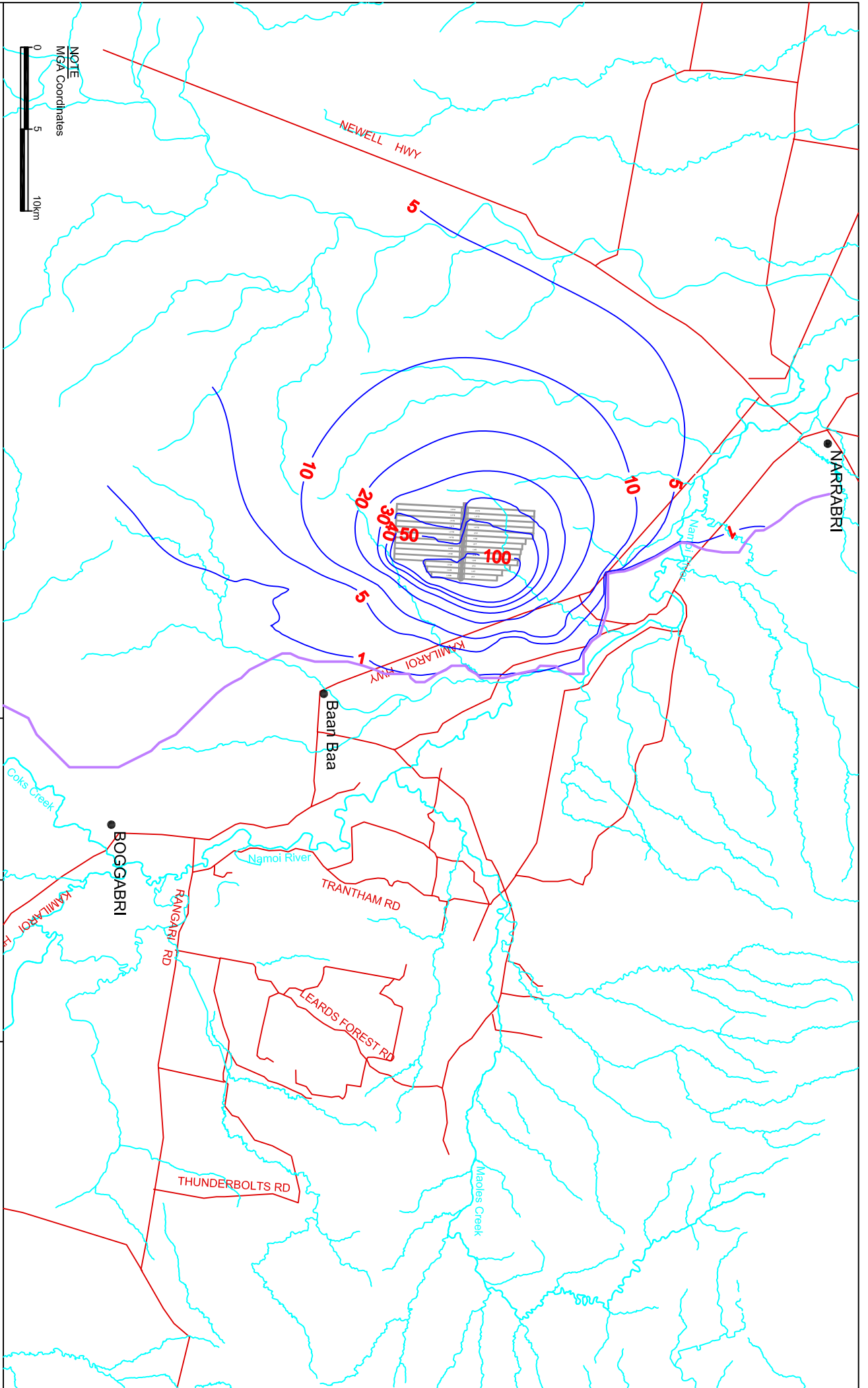
Rev: 0

Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Digby Formation (Layer 8)

aquaterra

Appendix J



NOTE
MGA Coordinates



Legend

- Narrabri Underground Outline
- Roads or tracks
- Surface Water Drainage (Stream/Creek)
- Predicted Groundwater Drawdowns (m AHD)
- Hodkissons Seam Sub Crop Margin

Date: 14 August 2009

Initials: PZ

Drawing No: S28-042c

Scale: As Shown

Job No: S28

Rev: C

Narrabri Coal Pty Ltd

Drawdown after 29 Year in
Hodkissons Seam (Layer 9)

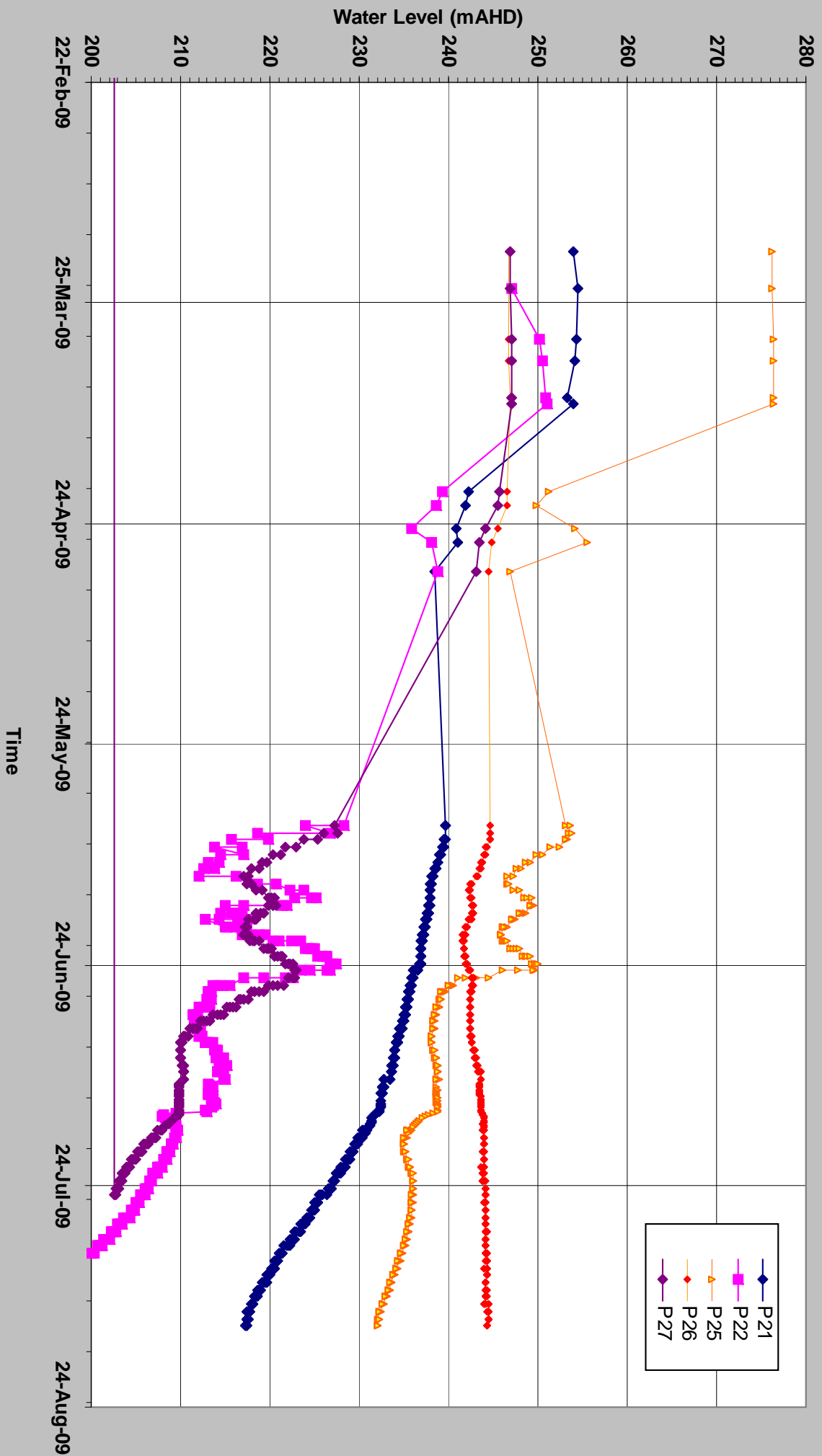
aquaterra

Appendix J

APPENDIX K

VIBRATING WIRE PIEZOMETER RESPONSE TO INSEAM GAS TESTING

HYDROGRAPHS - P21, P22, P25, P26 and P27



Date: 21 August 2009 Scale: As Shown

Initials: PZ Job No: S28

Drawing No: S28-086 Rev: 0

Narrabri Coal Pty Ltd

Vibrating Wire Piezometer Response to
Inseam Gas Testing

